Water Loss Management



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Water Loss Management

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Introduction

Water Loss Management



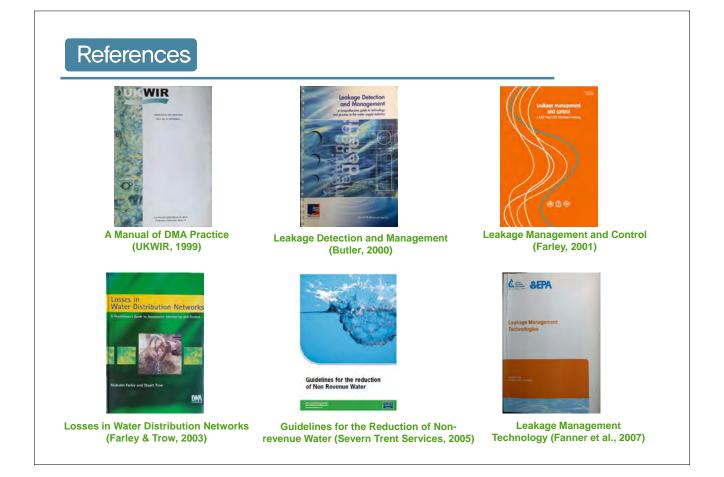
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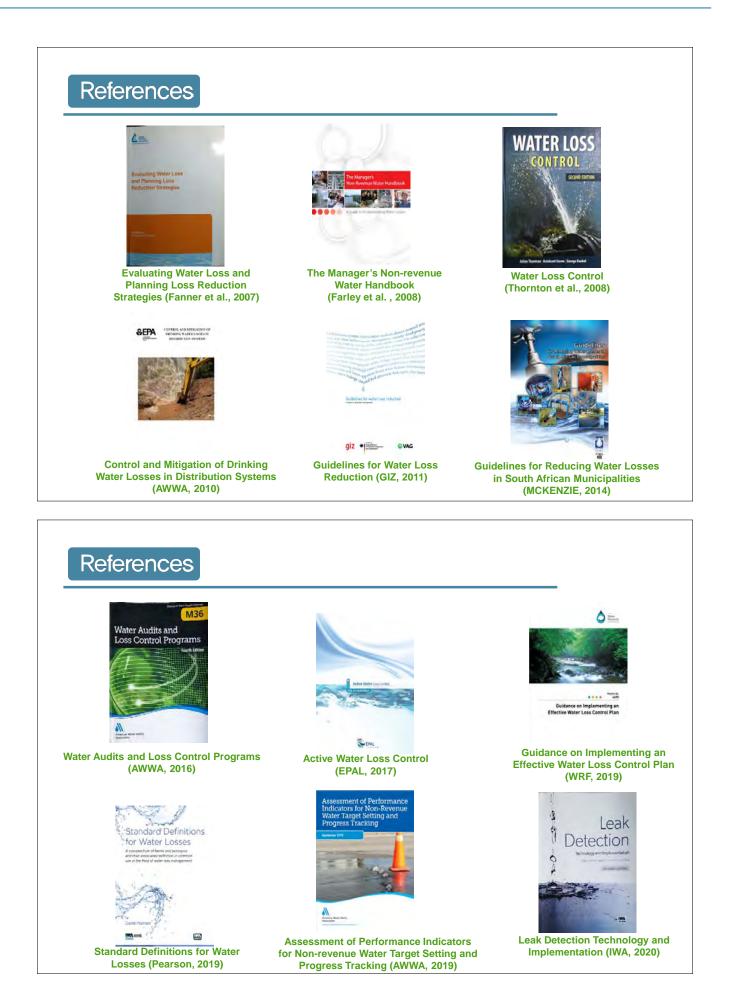
Aims & Objectives

Aims of the course:

Provide trainees with basic knowledge before proceeding with the course on water loss management

- Objectives: To enhance the trainees' understanding of:
 - (1) Vicious and virtuous cycles in water loss management;
 - (2) Types of water loss;
 - (3) Characteristics of water loss.





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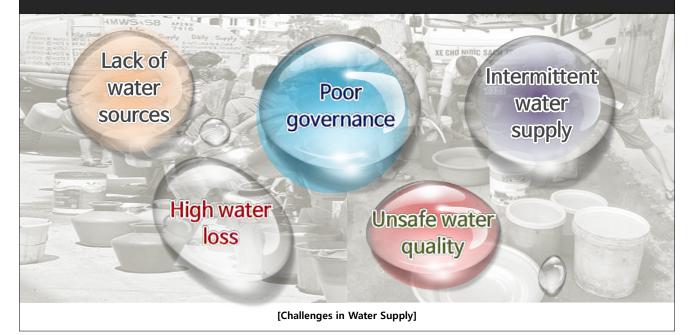
- 1. Overview
- 2. Challenges, Impacts, Results, and Improvement
- 3. Types of Water Loss
- 4. Volume of Water Lost
- 5. Visible and Invisible Leakage
- 6. Concept of Non-revenue Water (NRW)

Management



1.1 Background

 Challenges in water supply: The common water supply problems in several cities are related to the sources and use of raw water, intermittent supply, high water loss, and quality of tap water at the consumers' end



1.1 Background

 Problems of water loss: Water loss remains a major concern for all water supply systems due to wastage, technical burdens, contamination, and revenue losses to water utilities

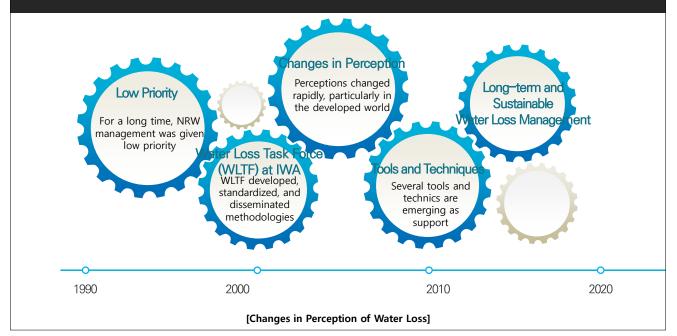


[Main Problems caused by Water Loss]

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1.2 Changes in Perception of Water Loss

 Changes in Perception: Governments and water utilities have realized that reducing water loss is critical, thus leading to the rapid change in the perception of water loss management



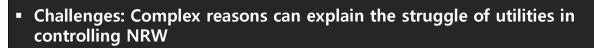
1.3 Stakeholders Collaborative governance: A water association, individual local water utilities, or the public sector can act as the driving force behind the introduction of water loss control programs · Owners of water companies Board of directors (or MDBs Water decision-makers) Bondholders Investors utility · Technical directors Engineers Municipal customers Collaborative Water association Groups of Customers Commercial customers Professions associated with Governance Professionals Industrial customers the water sector · Policy makers from national water ministries or authorities Policy Municipal governments (local) Economic regulators Government Provincial governments (local) • Environment agencies makers Central governments Department of the environment [Stakeholders in Water Loss Management]

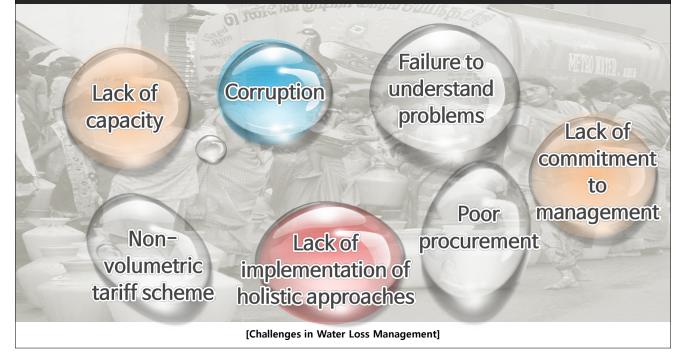






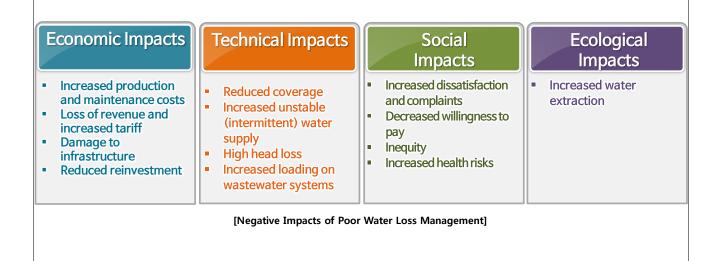
2.2 Challenges in Water Loss Management





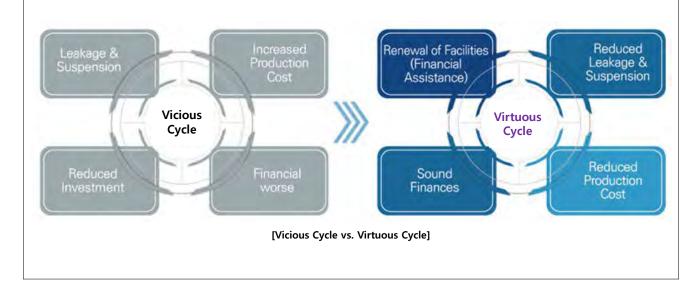
2.3 Impacts of Poor Water Loss Management

- Impacts: In the operation of water supply systems, water losses are a clear obstacle to sustainability, as evidenced by the following potential impacts
 - NRW exerts a serious impact on the financial viability of water utilities through loss of revenue, loss of water resources, and increased operational costs



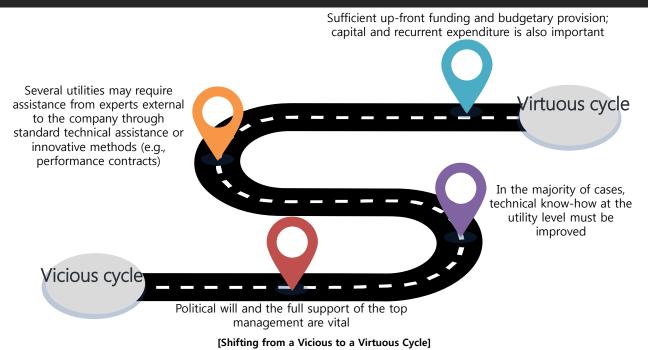
2.3 Results of Poor Water Loss Management

 Vicious cycle: Water collection, treatment, and distribution costs increase, whereas water sales decrease with the loss of a utility's product (treated water). Moreover, substantial capital expenditure programs are frequently promoted to meet the ever-increasing demand. In other words, the utility enters a vicious cycle that does not address the core problem



2.4 Improvement in Poor Water Loss Management

 Shifting from a vicious to a virtuous cycle: Many actions are required to enable this transformation



3. Types of Water Loss

3.1 Terminology

3.2 Classification of Real Loss

3.3 Classification of Apparent Loss

3.1 Terminology

Terms: Standardized terminologies are a precondition for water loss management

- Terminologies are well explained and defined in Standard Definitions for Water Losses (Pearson, 2019)

Water loss

Total water loss pertains to the difference between the amount of water produced and amount billed or consumed

Leakage

Leakage is one of the components of the total water loss in a network and comprises physical losses from pipes, joints, and fittings and from overflowing service reservoirs

Water waste

Deliberate waste (e.g., standpipe vandalism and tap left "open" permanently in areas with intermittent supply to fill vessels when supply returns, which then overflows)

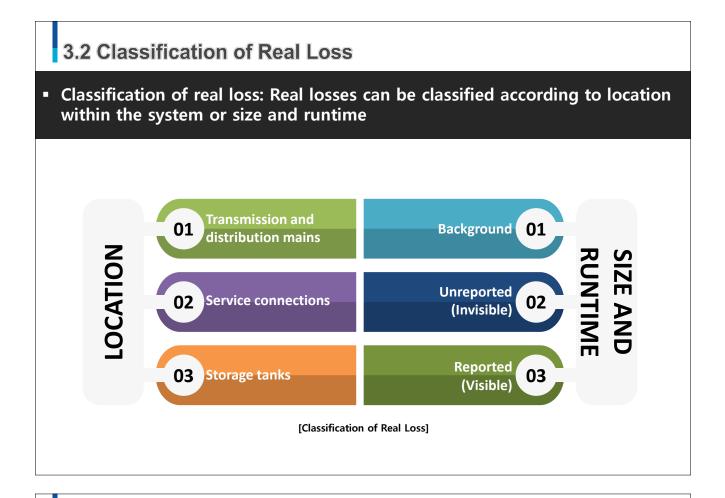
Real loss

Real loss denotes loss from the pressurized system and from the utility's service reservoirs to the point of supply

Apparent loss

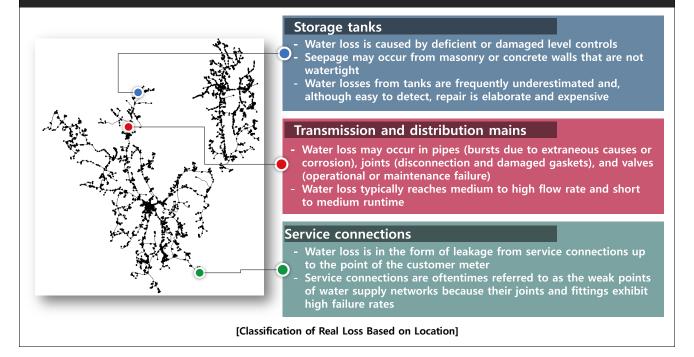
Apparent loss includes all types of inaccuracies associated with customer metering, data handling errors, and unauthorized consumption

[Definition of Terminologies in Water Loss Management]



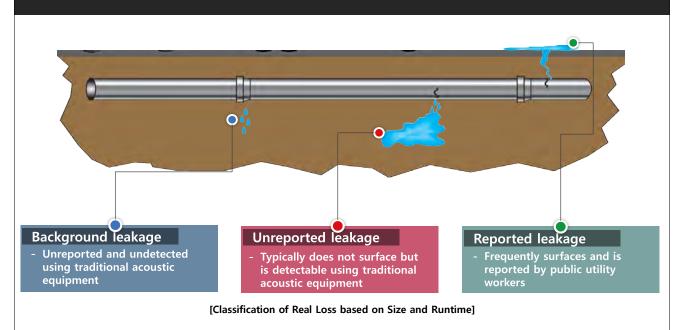
3.2 Classification of Real Loss

 Location: This classification is typically used when the IWA water balance is calculated



3.2 Classification of Real Loss

Size and runtime: This classification is used when minimum night flow analysis is implemented



3.2 Classification of Real Loss

 Background leakage: Background leaks are basically extremely small leaks that tend to run unnoticed, which can last for years and may not create any problems until they eventually grow into large leaks

Background Leakage

Sharing: "Background leaks frequently make up the major proportion of real water loss due to their great number and long runtimes."

Difficult to repair: "Even if background leaks could be detected, repairing them would not be cost effective because the costs involved would be orders of magnitude higher than the value of the water that could be saved."

Control: "Although background leakage can never be eliminated completely, it can be controlled to a certain extent through pressure management or, as a last resort, by pipe replacement"



[Background Leakage]

3.2 Classification of Real Loss

Burst leakage (unreported & reported): The normally accepted definition of burst leakage is that they amount to more than 250 l/h at a 50-m pressure

Reported and unreported leakage

Leakage amount: "Burst leakage occurs on mains and connections. Such leaks on mains tend to be spectacular and can range in size from below 2 m³/h to more than 1,000 m³/h, although the average leak in mains is likely between 2 m³/h and 4 m³/h."

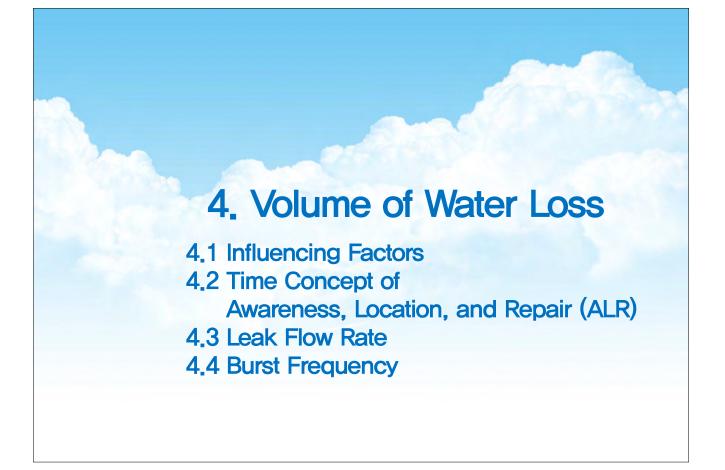
Definition: "By definition, unreported or hidden leaks have flow rates greater than of 250 l/h at 50 m pressure but do not appear at the surface due to unfavorable conditions."

Methods for detection: "Methods for leak detection primarily aim to locate unreported leakage, which accounts for the majority of water loss due to leakage in many water distribution systems."



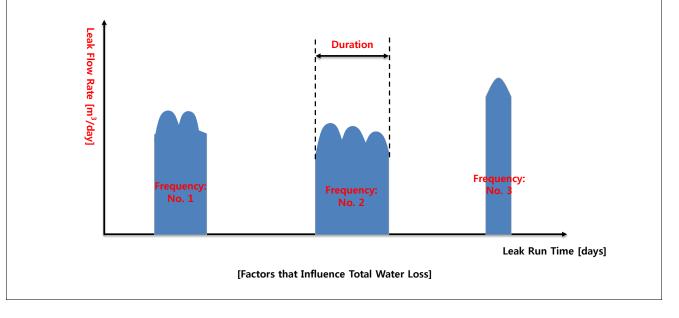
[Reported and Unreported Leakage]

3.3 Classification of Apparent Loss Classification of apparent loss: Apparent loss consists of three main elements, namely, meter inaccuracies, data handling errors, and unauthorized consumption Management Archiving Accounting Transmitting Reading Meter Collection Water flow Billing AMR Platform Connectivity AMI System SM Meter inaccuracies Unauthorized consumption Data handling errors The volume by which meters under-record the true volume Unauthorized consumption Errors in handling data on represents water theft, unlawful systematic consumption, meter convention, and meter reader corruption consumed by customers particularly in the customer billing system [Classification of Apparent Loss]



4.1 Influencing Factors

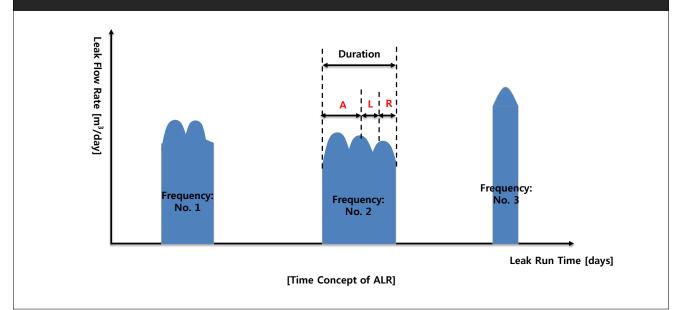
- Influencing factors: Leak flow rate, duration, and burst frequency influence the volume of water loss
 - Therefore, the volume of water loss is mainly dependent on the characteristics of the pipe network and leak detection and repair policies practiced by a company



4.2 Duration: ALR Time Concept

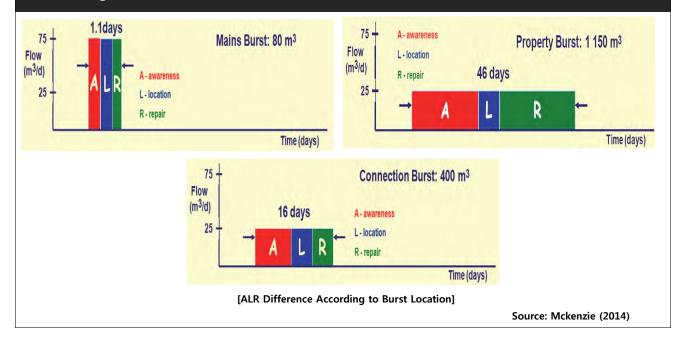
• ALR concept: Leak duration (runtime) consists of three time components:

- Awareness: time until the utility becomes aware of the existence of a leak
- Location: time spent to precisely locate the leak to enable the issuance of a repair job order
- Repair: time between the issuance of the repair job order and completion of the repair



4.2 Duration: ALR Time Concept

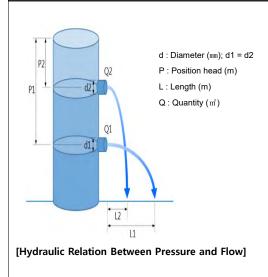
 ALR difference: If awareness time largely determines the volume of water loss from a pipe burst, then utilities should implement a strategy for reducing awareness time



4.3 Leak Flow Rate

Pressure versus flow rate: The flow rate typically increases with the exponential growth in pressure

- Leakage discharge tests show that water loss from a single circular hole with a diameter of 6 mm in a distribution pipe with a 50-m pressure equals 1.8 m³/h (1,300 m³/month)



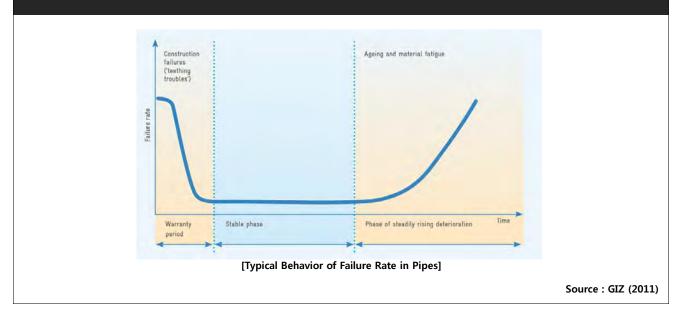
	Orifice		Leakage flow rate		
	(mm)	[l/min]	[l/hour]	[m³/day]	(m*/month)
Ì	0.5	0.33	20.00	0.48	14.40
	1.0	0.97	58.00	1.39	41.60
	1.5	1.82	110.00	2.64	79.00
	2.0	3.16	190.00	4.56	136.00
	3.0	8.15	490.00	11.75	351.00
	4.0	14.80	890.00	21.40	640.00
	5.0	22.30	1,340.00	32.00	690.00
	6.0	30.00	1,800.00	43.20	1,300.00
	7.0	39.30	2,360.00	56.80	1,700.00

[Leakage Flow Rate for Circular Holes at a 50-m Pressure]

Source: GIZ (2011)

4.4 Burst Frequency

 Linear relation: The amount of leakage is linearly related to burst frequency. Experience shows that failures accumulate at the beginning (construction failure) and end (i.e., material fatigue and corrosion) of the useful life of discrete pipe sections, such as a bathtub curve



5. Visible and Invisible Leakage

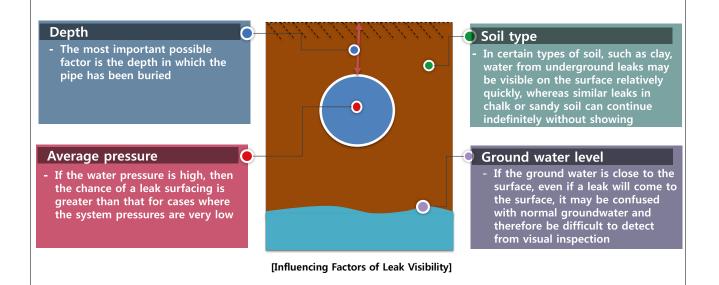
- 5.1 Visible and Invisible Leaks
- **5.2 Influencing Factors**
- 5.3 Two Perspectives on Invisible Leakage

5.1 Visible and Invisible Leaks

- Visible leakage will appear as clear water on the surface. If pipes are pressurized, then water bubbling up to the surface may be detectable.
 - A possible scenario is that visible leaks have been running for a relatively long time, and vegetation may indicate the presence of a leak despite the absence of water on the surface.
- Controversial subject: The actual distinction between visible and invisible leaks is a debatable point and may vary according to area
- Active leak control (ALC) for detecting invisible leaks: If invisible leaks are dominant, then active leakage policies should be implemented.

5.2 Influencing Factors

Influencing factors of visibility: To understand why some leaks are visible on the surface, whereas others remain underground and invisible, the following factors should be considered.



5.3 Two Perspectives on Invisible Leakage

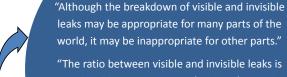
- Two perspectives: Major water distribution experts hold two major views on how the problem should be considered
- Confirmation: The only method for determining leaks is to undertake a pilot study in a small area before embarking on a city-wide leak location exercise

Visible leak: dominant

"In general, approximately 90% of water physically lost from leaks are not visible on the

"Oftentimes, undetected leaks can be relatively large, such as those that run directly into a sewer or a drain."

"Water loss from large-diameter pipes can be significant in low-pressure systems, where leaks may not be visible on the surface and remain unnoticed for many years ."



leaks may be appropriate for many parts of the world, it may be inappropriate for other parts."

"The ratio between visible and invisible leaks is not 10 to 90. In many cases, the virtual opposite appears to be true."

Invisible leak: dominant

"This opinion will draw considerable criticism especially from those selling leak-location equipment."

[Two Views on Invisible Leak]

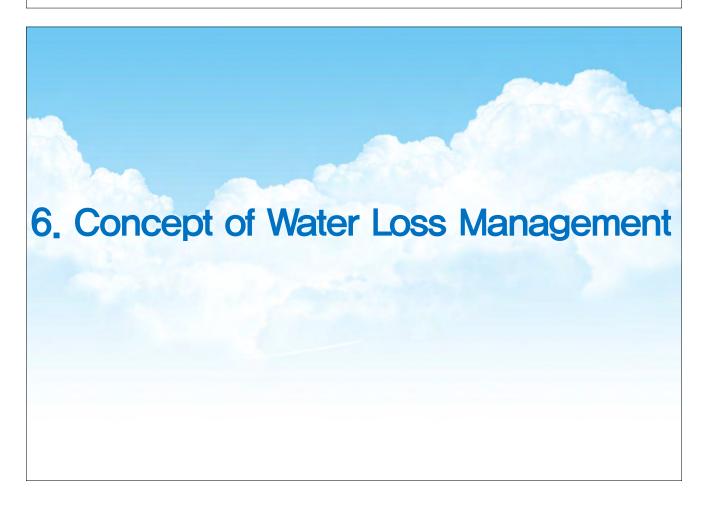
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5.3 Two Perspectives on Invisible Leakage



[Cases of Visible Leakage]

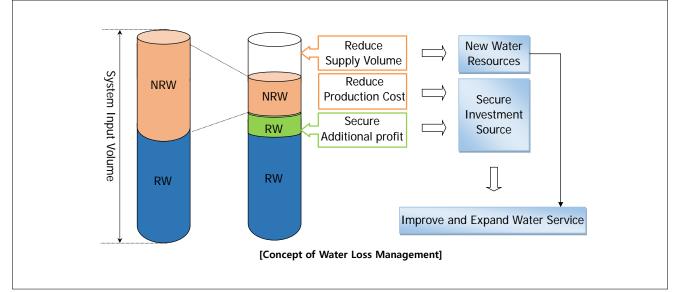
Courtesy of K-water



6. Concept of Water Loss Management

Water loss management: By reducing NRW, water utility services can reduce the supply volume and secure additional profit

- Reducing excessive real water loss results in the greater amount of water available for consumption and postpones the need for investing in new sources
- Similarly, reducing commercial losses generates additional revenues with a short payback period

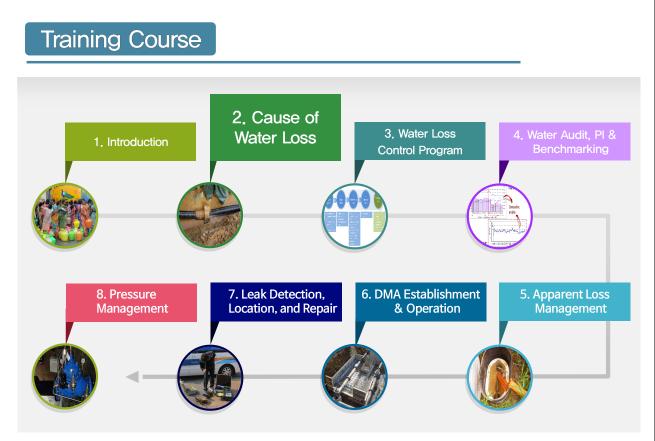




Cause of Water Losses

Water Loss Management



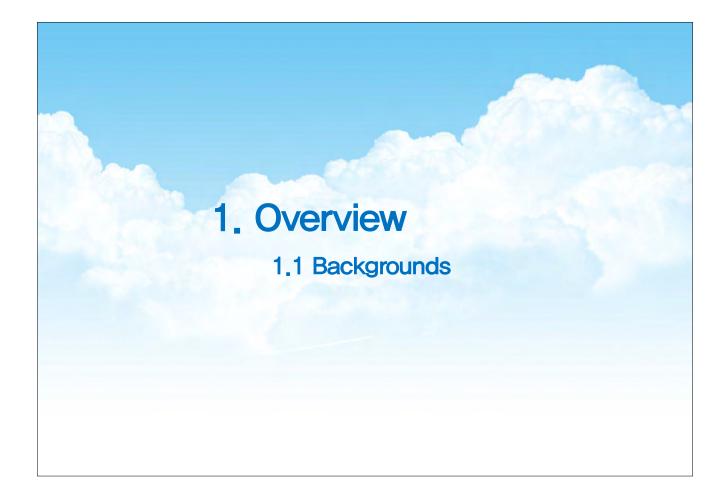


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Aims & Objectives
 The aims of the course are to:
 Promote among trainees a good understanding of the causes of water loss according to the types
(2) Enable trainees to identify the main leakage causes of their system
The objectives are that trainees will understand:
(1) Causes of real loss(2) Causes of apparent loss

Contents

- 1. Overview
- 2. Causes of Real Loss
- 3. Causes of Apparent Loss

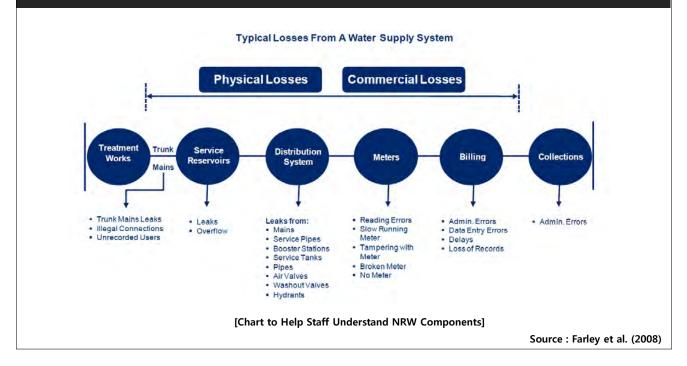


1.1 Backgrounds

- (Need understanding before intervention) Before deciding which intervention methods are appropriate in water loss management, a water utility has to understand which causes attribute to the water loss in their water supply systems
- (Systematic or technical) The causes of real loss and apparent loss can be mainly classified into systematic reason and technical reason
- (Compound influence) It is evident that various causes have a complex effect on the leakage of the system
- (Limitation of elimination) While apparent losses can be nearly totally eliminated, a certain level of real losses (i.e. unavoidable real losses) will always remain in any water supply system

1.1 Backgrounds

(Water losses location) Real losses are highly related with water supply pipeline and apparent losses are mainly related with billing process



1.1 Backgrounds

- (Dominant part in water losses) In developed countries, real losses usually represent the most important component of water losses
 - However, in developing and emerging countries losses due to illegal connections, metering and accounting errors (i.e. apparent losses) may often be of major significance to water utilities

 (Misconception) When people think about leakage, they normally think of big and spectacular pipe burst

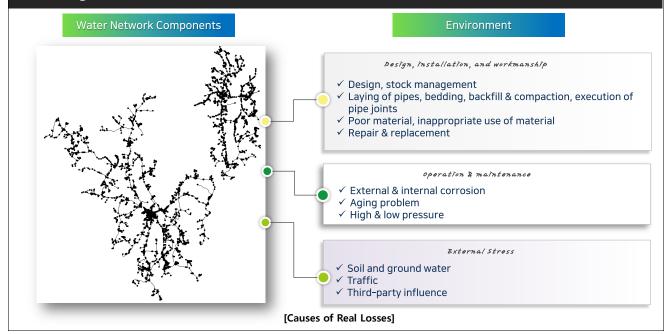
- They often cause a lot of damage but are insignificant in volume compared to all the other leaks that do not come to the surface
- Normally most of real loss can not be seen on the surface

 (Overlooking apparent losses) In contrast to real losses, the apparent losses are not visible, which lead many water utilities to overlook apparent losses and concentrate instead on real losses

2. Causes of Real Losses 2.1 Causes of Real Losses 2.2 Pipe Deterioration Mechanism 2.3 Real Losses at Pipes & Joints, Valves, & Fittings 2.4 Real Losses at Reservoirs, Storage Tanks & Pumps

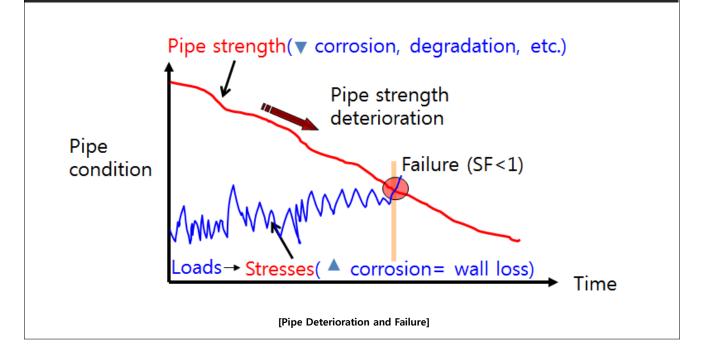
2.1 Causes of Real Losses

 (Causes of real losses) The multitude of active and passive interactions between water network component and their environment frequently lead to damage and leaks



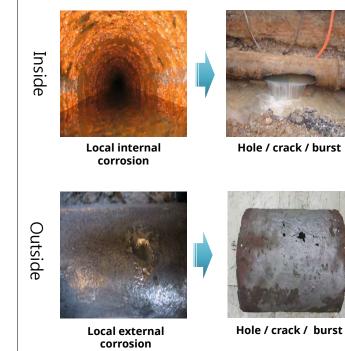
2.2 Pipe Deterioration Mechanism

 (Pipe deterioration) Over time, pipe strength are reduced due to corrosion or degradation, it will be resulted in physical failure



2.2 Pipe Deterioration Mechanism

(1) Unlined CIP/ DCIP



2.2 Pipe Deterioration Mechanism

(2) CML-DCIP









CML is changed to sand



100% CML neutralization Local internal corrosion



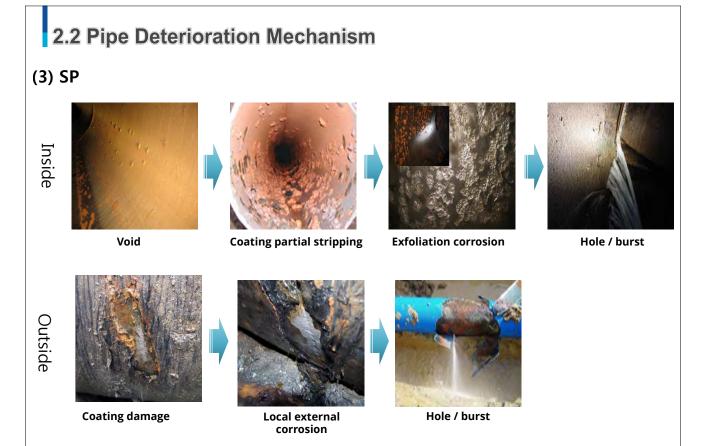
Hole / crack/ burst



Local external corrosion

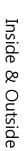


Hole / crack / burst



2.2 Pipe Deterioration Mechanism

(4) Plastic (PE, PVC) Pipe







Deformation

Degradation



Burst

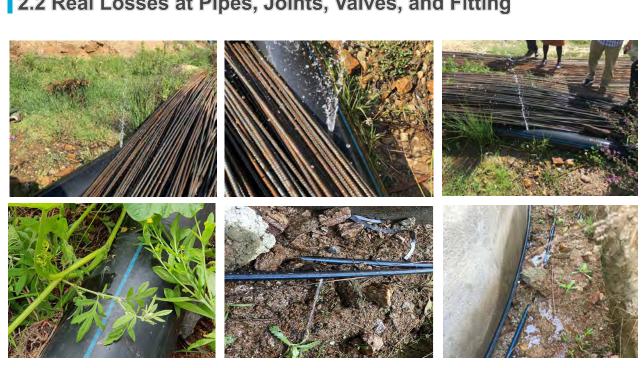
2.2 Real Losses at Pipes, Joints, Valves, and Fitting

 (Design, installation, and maintenance workmanship) Inappropriate design, installation and maintenance workmanship can cause the real losses in the water network

Causes & factors influencing water losses		Details		
Main	Sub			
Design, installation, and workmanship	Design	 Mistakes made during the planning phase may influence leakage from pipes, such as the incorrect material choice, insufficient dimensioning for the actual pressure, inadequate corrosion protection measures or incorrect alignment 		

[Pipe Burst caused by Two Pipes Contact]

Courtesy of K-water



[Leakage in Exposed pipe]

Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting



[Leakage in Unorganized Service Connection]

Causes & factors influencing water losses		Details
Main	Sub	
Design, installation, and workmanship	Storage of pipes	 Improper storage may damage pipes even before installation Grass roots can penetrate the bituminous coating of steel pipes Dragging PE pipes over concrete surfaces or stones will cause chambers which are more susceptible to future leakage Extended sunlight exposure causes PE pies to become brittle
	Laying of pipes	 The quality of workmanship when laying the pipes is one of the most important factors influencing the leakage
	Bedding	 The selection of inappropriate material for pipe bedding is a frequent cause of damage * Coarse or rocky bedding material spoil external coatings on steel or cast iron pipes and support corrosion * Longitudinal and spiral cracks may appear in PE and PVC pipes as a result of stony bedding materials
		[Leaks Caused By Inappropriate Bedding]
		Courtesy of K-wate

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

losses	influencing water	Details
Main	Sub	
Design, installation, and workmanship	Backfill & compaction	 Insufficient backfill and compaction of the pipe trench may cause subsidence * These uncontrolled soil movements can also trigger socket disconnection or pipe rupture
- 227		N. Ster

[Damage Caused by Casting Concrete above the Pipe]



Causes & factors influencing water losses		Details
Main	Sub	
Design, installation, and workmanship	Improper installation of pipe and accessories	 Unprofessional execution of pipe joints is a further reason for leakage Welded steel pipes often lack proper internal and external corrosion protection along the weld seam If unskilled or poorly trained workers execute the butt fusion in PE pipes, it frequently contains defects due to insufficient heating and pressing of the pipe ends Leakage can also occur if socket pipes exceed the maximum permitted angular bending or if water hammer and high pressure affect joints that are not force locked
		Image: Contract of the second secon
		Courtesy of K-water

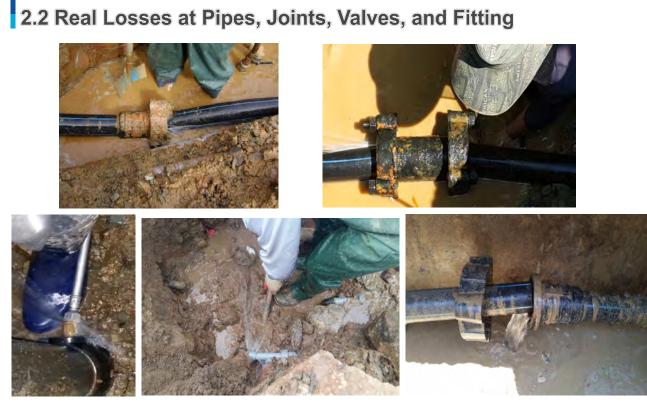
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[Leakage at the Unproperly Deformed Pipe using Heat in the Field]

Courtesy of K-water





[Leakage Caused by Excessive Joint Deflection]

Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

Causes & factors influencing water losses		Details
Main	Sub	
Design, installation, and workmanship	Poor material	 Material failures can be happen because of manufacturing defects * Insufficient wall thickness * Poor internal and external protection (lining and coating) * Unreliable materials of pipes, joints, fittings, etc.
	Inappropriate use of certain material (suitability, cement neutralization)	 There is a great variation in the suitability of service and supply pipe materials (lead, galvanized iron, cooper, polyethylene, etc.) * Soft water with a high carbonic acid (CO2) content and low calcium concentrations, or with high sulphate concentrations are known to affect concrete aggressively
	1 P	
		[Unreliable Casting Materials in DCIP]





[Poor Welding in SP]

Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting



[Pipe Burst Caused by Poor Material DCIP and Stress Concentration]

Courtesy of K-water



[Pipe Burst Cased by Poor Material in DCIP]



[Poor PE Joints]

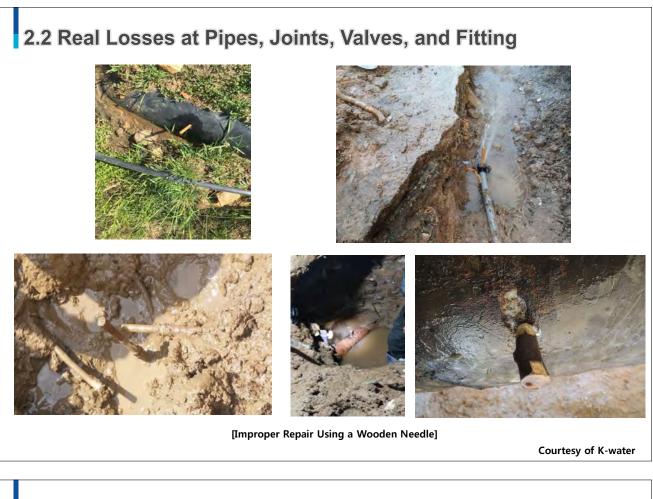
Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

Causes & factors influencing water losses		Details
Main	Sub	
Design, installation, and workmanship	Inappropriate repair and replacement	 If improper repair or replacement of failure pipes or fittings can cause another leakages



[Improper Repair Using Tape]





[Improper Repair Using a String or Rubber Band]

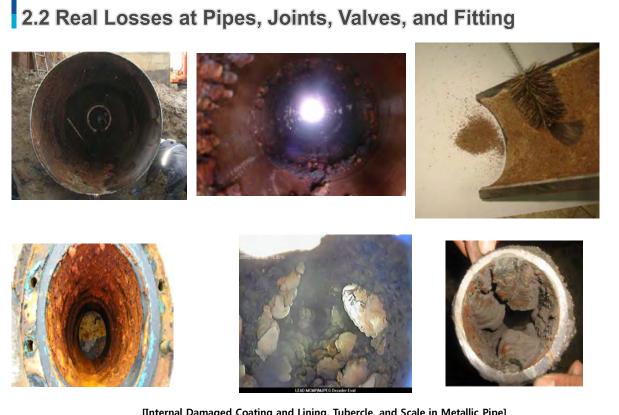
 (Operation & maintenance) The use of poor material, corrosion, infrastructure aging problem, and high & negative pressure causes real losses

	 Details All metallic pipes are exposed to physical and electro-chemical corrosion and it will reduce the wall thickness and decrease the pipe's ability to withstand water pressure and external stress The most common causes of corrosion are aggressive water and soil as well as stray current * Internal corrosion is usually more sever in soft (acidic) waters from upland sources * Aggressive soils may cause external corrosion because of differing level of dissolved salts, oxygen, moisture, pH, and bacterial activity, leading to corrosion currents in the metal
Sub	
Condition (external & internal corrosion)	
	Condition (external & internal





[Leakage Caused by Corrosion in Metallic Pipe]



[Internal Damaged Coating and Lining, Tubercle, and Scale in Metallic Pipe]

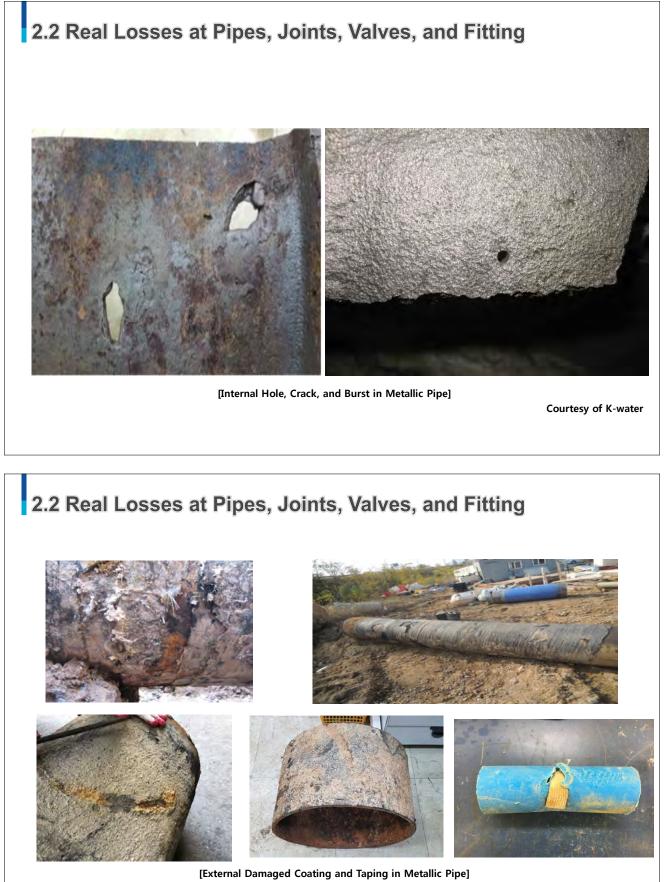
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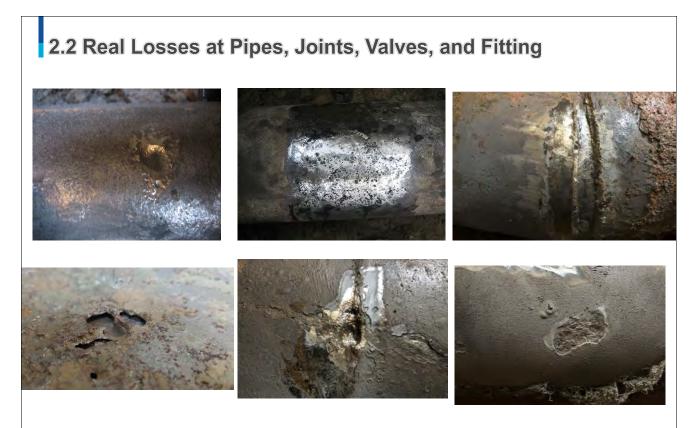
2.2 Real Losses at Pipes, Joints, Valves, and Fitting



Courtesy of K-water

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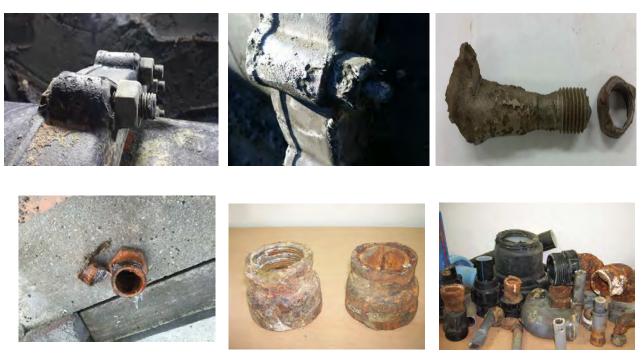


[External Corrosion in Metallic Pipe]

Courtesy of K-water



[External Hole, Crack, and Burst in Metallic Pipe]



[Corrosion of Pipe Accessories]

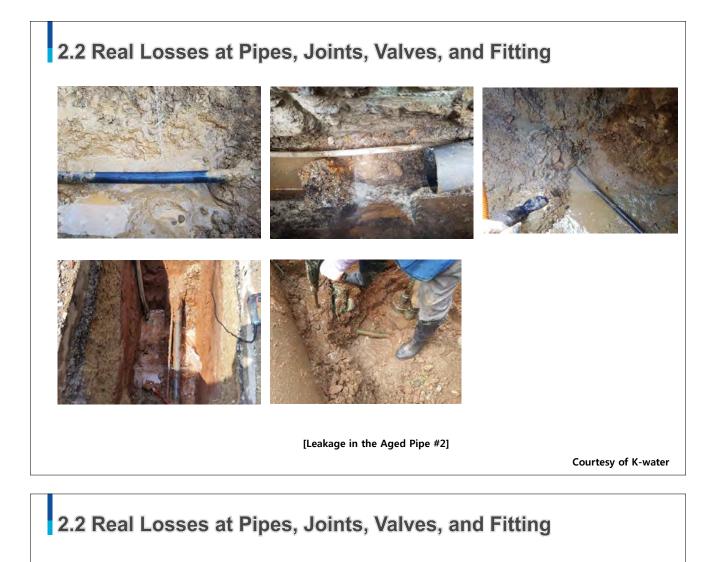
Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

Causes & factors influencing water losses		Details
Main	Sub	
O&M	Aging Problems	 Many factors influencing leakage are age-dependent Consequently, the age of a pipe section can appear to be the most significant factor for leakage Nevertheless, age is not necessarily a factor, if the pipe has been carefully designed and installed, maintenance is carried out at regular intervals and external conditions are favorable



[Leakage in the Aged Pipe #1]





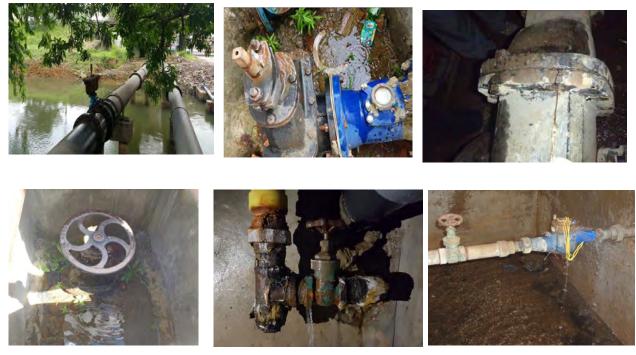
[Broken Pipe]



[Leakage in the Aged Joint #1]

Courtesy of K-water



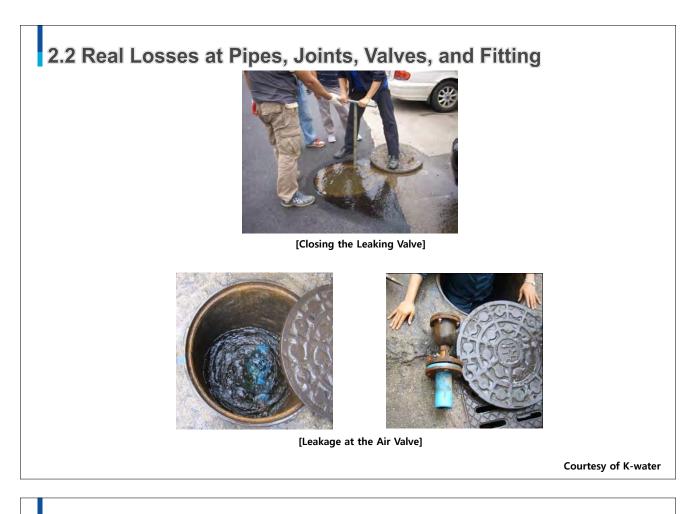


[Leakage at the Aged Valve #1]

Courtesy of K-water

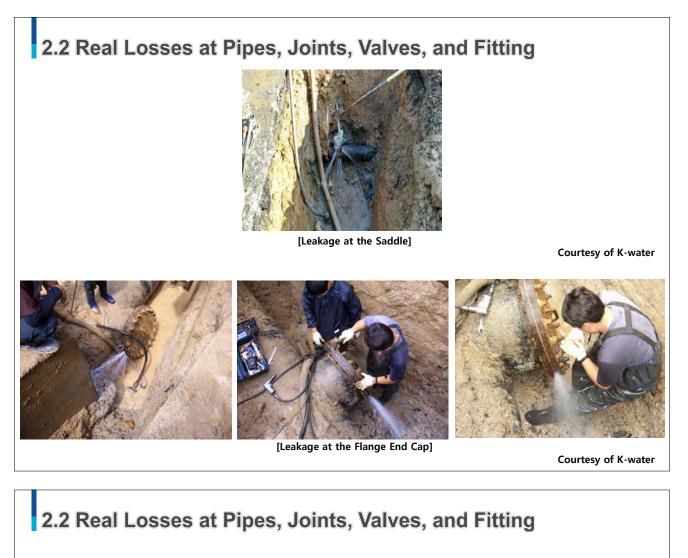
2.2 Real Losses at Pipes, Joints, Valves, and Fitting







[Leakage at the Meter Box]

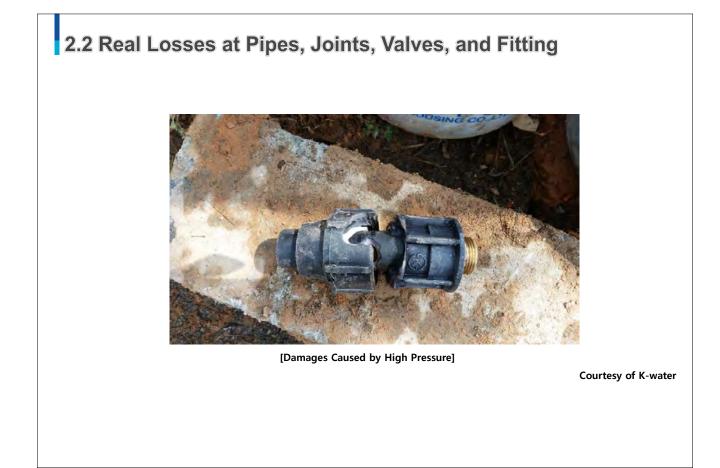




Causes & factors influencing water losses		Details
Main	Sub	
0&M	Pressure: high Pressure	 Increasing pressure will result in a higher flow rate from existing leaks High pressure will increase the pipe burst frequency



[High Leak Flow Rate in High Pressure]



Causes & factors influencing water losses		Details
Main	Sub	
0&M	Pressure: low pressure	 Low pressure may complicate leak detection efforts because the water is less likely to reach the surface Decreased noise levels from the leak in low pressure impede acoustic leak detection and may cause longer leak awareness time
	Pressure: pressure cycling	- Pressure cycling in the network due to bad operation may lead to material fatigue and thus to leakage, mainly in plastic pipes
	Pressure: water hammer	 Pressure surge (water hammer) can happen when a pump is tripped, or a valve is opened or closed too quickly This can cause pipe fractures, disconnect joints and damage valves and fittings, move thrust blocks, or damage the socket hence leading to leakage There is also some evidence that surge can cause pipes to flex and move against rocks resulting in local stress concentrations, and sometimes pipe failure

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

(External load) External load caused by soil, ground water, and traffic can damage the pipes

- Unfortunately, water utilities can not control the weather, geologic conditions, or even traffic to any great extent

Main	Sub	
External force	Soil and groundwater : soil aggressiveness	 Cohesive soil may negatively affect the external corrosion of metallic pipes differing levels of dissolved salts, oxygen, moisture, pH and bacterial activity * Most non-cohesive soils are not aggressive
	Soil and groundwater : soil movement	 Soil movement may cause pipe joints to disconnect and pipes to rupture, or resul in localized stress concentrations within the pipe leading to failure Soil movement is caused by changes in the temperature and moisture content, heavy frost as well as subsidence prompted by incorrect pipe bedding, mining activities or earthquakes Construction work, increased surface loads or pipe repair work may also trigger soil movement



[Loss and Restoration of Pipelines due to Heavy Rain]

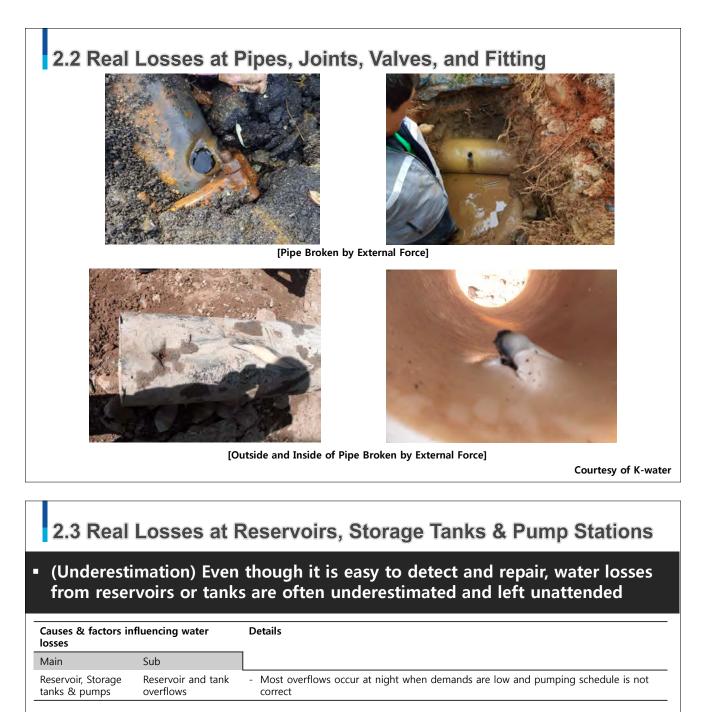
Courtesy of K-water

2.2 Real Losses at Pipes, Joints, Valves, and Fitting

Causes & factors influencing water losses		Details
Main	Sub	
External force	Traffic load	- More vehicles and the high loading trucks place an additional burden on the pipes
	Pavement	 The visible leaks may appear far away from the damage due to concrete and asphalt pavements, thus hampering leak location efforts
	Stray current	- Stray currents aggravate the external corrosion of metallic pipes
	Accidental or deliberate damage	 Accidents may be happen during the construction or other utilities installation if workers have wrong information on water pipelines



[Leakage from Pipe Broken by External Force]





[Overflow at Distribution Reservoir]

2.3 Real Losses at Reservoirs, Storage Tanks & Pump Stations

Causes & factors influencing water losses		Details
Main	Sub	
Reservoir, Storage tanks & pumps	Structure damage of reservoirs and tanks	 Structural damage involves cracks, holes or delamination at reservoir(or tank) walls o floors, leakage due to inferior concrete quality, as well as sealings and pie penetrations that are not watertight

[Delamination and Leakage at Distribution Reservoir Wall]

Courtesy of K-water

2.3 Real Losses at Reservoirs, Storage Tanks & Pump Stations

Causes & factors influencing water losses		Details
Main	Sub	
Reservoir, Storage tanks & pumps	Defective pump	 Water losses from pumps are usually caused by defective pump shaft seals The amount of leakage is negligible in most cases, but flooding pump chambers and electric equipment is a nuisance and should be avoided by undertaking proper maintenance



[Leakage at the Valve and joint in Pump Station]

3. Causes of Apparent Losses

- **3.1 Causes of Apparent Losses**
- **3.2 Metering Inaccuracies**
- **3.3 Systematic Data Handling Error**
- 3.4 Unauthorized Consumption

3.1 Causes of Apparent Losses

 (Apparent losses) Apparent losses includes meter inaccuracy, data handling error, and unauthorized consumption

Meter Inaccuracy

- Lack of meter maintenance
- Registering error
- Improper meter selection
- Incorrect installation
- Deterioration

Data Handling error

- Administrative error
- Meter reading error
- Data transfer error
- Distorted customer consumption
- Estimated volume
- Poor customer account management

Poor billing system
 [Main Causes of Three Apparent Losses Types]

Unauthorized

- Illegation
- Meter by-passing
- Meter tampering
- Theft at hydrants or fountains
- Corruption of meter reader

(Meter inaccuracies) Inaccurate meters tend to under-register water consumption which reads to reduced sales and therefore reduced revenue

Causes & factors influencing water losses		Details
Main	Sub	
Lack of periodic meter testing and maintenance	Lack of periodic meter testing and maintenance	 Contributing to poor meter problem is the lack of good quality meter testing facilities, especially when it comes to larger diameter meters, and the lack of experience in how to best utilize such facilities
Registering error (physical capacity error)	Under-registering: aging (wear out)	- It is recognized that mechanical meters tend to have a gradual tendency for under- registration throughout their life cycle
	Under-registering: faulty meter	- Water volume can not exactly measured in stopped or faulty meter
	Under-registering, Low flow	 Under-recording can occur when flow rate is below the at low flows such as when a flow rate is below the minimum flow (Q1) Turbine meters are designed to capture continuous moderate and high flows, but if the user has periodic lower flows, exact flow rate can not be measured
	Over-registering, Air	 If a fluid other than water(e.g., air) occupies part of the volume in the metering chamber, the metering error can be quite severe and rapid wear of moving parts may occur In intermittent water supply, customer meters will register a certain volume of air when the water supply is first turned on

3.2 Metering Inaccuracies



[Stopped Bulk Meter]

Causes & factors influencing water losses		Details
Main	Sub	
Improper meter (intrinsic error)	Size	 Sometimes, the selection of water meter size is based on availability of meters in stock and the size of the service pipe rather than on a customer's consumption pattern In the past, it was common to size customer service connections and meters based on the peak flow rates that the meter was expected to encounter Oversized meters tend to under-register under low flow conditions and may lead to larger differences between the recorded volume and the water that is actually consumed
	type	 Utilities should seek to select appropriate meter types and prepare tailored specifications * This can prove difficult, especially where procurement laws and regulations encourage purchasing the cheapest products on the market
		[Improper Meter Size]
		Courtesy of K-water

3.2 Metering Inaccuracies

Causes & factors influencing water losses		Details
Main	Sub	
Incorrect installation of v meters	vater	 There are various requirement regarding the length of straight pipe before and after each type of meter The supplier will provide details of these requirements for each meter When meter readers identify the meter, it should be easy to determine which meter belongs to which properties All meter should be installed above ground and located where they can be audited easily, including by the meter readers during their regular rounds
		Improper Meter Installation] Courtesy of K-water



[Difficulty in Finding Correct Customer Meter]

Courtesy of K-water



[Difficulty in Assessing Customer Meter]

Causes & factors influencing water losses		Details
Main	Sub	
Deterioration	Harsh - Extreme heat or cold affecting water meters environmental condition	- Extreme heat or cold affecting water meters
	Suspended particles	 Water quality and suspended particles (sedimentation) can greatly degrade water meter accuracy * In some cases, it produce a blockage of the meter * Because suspended particles often enter the distribution system during repair work





[Debris at Strainer or Rotating Part]



Courtesy of K-water

3.2 Metering Inaccuracies

Causes & factors influencing water losses		Details
Main	Sub	
Deterioration	Poor maintenance	- Poor maintenance will not only encourage inaccuracy but may shorten the life span of the meter



[Deteriorated Water Meter]

3.3 Systematic Data Handling Error

- (Data handling) Data handling errors include reading, transmitting, archiving, and reporting error
- (Data transferring process) The typical method of data handling and billing requires a meter reader to visit each property and read the customer meter
 - The data is then recorded by hand on a form, taken back to the office, given to the billing department, and typed into a billing system
 - A bill is then printed and mailed to the customer

Causes & factors influencing water losses		Details
Main	Sub	
Data handling error:	Meter reading error	 Errors can be easily negligence, laziness, or aging meters during the process of reading the meters and billing customers Sometimes lazy meter reader records estimated value and use it as a reading value Dirty dials, faulty meters, and jammed meters can also contribute to meter reading errors Incompetent or inexperienced meter readers may read the meter incorrectly or make simple errors, such as placing a decimal in the wrong place
	Data transfer error	- Errors during transferring written data to the office, AMR equipment failure, and data entry errors during meter change-outs can make the wrong data

3.3 Systematic Data Handling Error

Causes & factors influencing water losses		Details
Main	Sub	
Billing system error Distorted custom consumption		 There are many day-to-day processes in operating a billing system that have the potential to corrupt the integrity of the consumption data, depending on the design of the billing system It is important that water utility managers understand the working of the customer billing system with regard to consumption data integrity Most billing systems are not designed to retain the integrity of consumption data and they are designed to deliver accurate bills to customers and correctly account for the bills
	Estimated volume	- It may not be serious problem to correct estimated volume in billing system for billing purpose, however, it can cause a significant effect on water loss calculation
	Poor customer account management	 Unregister customer can contribute the distorted water audit results * Sometimes connections are made legally, but the billing department is not notified of the new connection, therefore the customer is never billed * Some accounts may not be activated, lost, or transferred erroneously

3.4 Unauthorized Consumption

 (Unauthorized consumption) Unauthorized consumption includes illegal connections, meter by-passing, meter tempering, illegal water withdrawal from hydrants, and corruption of meter reader

Causes & factors influencing water losses		Details
Main	Sub	
Illegal use	Illegal connection	 Illegal connections involve the physical installation of a connection to water distribution pipelines without the knowledge and approval of the water utility Illegal connections can occur during the installation of a new supply connection, or sometimes the customer's supply is cut off after non-payment and the customer cannot afford, or does not water to pay, to be reconnected Water theft can occur from large pipes as well as from household water connections
		Illegal Connection]
		Courtesy of K-water

3.4 Unauthorized Consumption

Causes & factors influencing water losses		Details
Main	Sub	
Illegal use	Meter by-passing	 Some customers try to reduce their water bill by using a meter bypass, which is an additional pipe installed around the meter * Sometimes, this bypass pipe is buried and very difficult to detect * Meter by-passing is usually committed by agricultural, industrial, or commercial purpose
	Meter tampering	 People sometimes temper with their meter to lower the measured volume * Customers may insert pins or other objects into the meter to disturb its moving parts * Some try to affect the readings of metal meters by attaching a strong magnet to it * In other cases, customers have boiled plastic meters trying to melt the internal plastic parts
	ССССКА (1927) С. В. 2015 (1927)	
		[An Illegal By-pass at a Water Meter] Courtesy of K-water

3.4 Unauthorized Consumption







[An Illegal By-pass for Irrigation]

Courtesy of K-water



[An Illegal By-pass for Construction]

Courtesy of K-water

3.4 Unauthorized Consumption

Causes & factors influencing water losses		Details
Main	Sub	
Illegal use	Theft at hydrants or community fountains	 People can use them illegally to fill tankers (normally at night) or as water supply connections to construction sites * The only legal use of fire hydrants is for fire fighting These high flows are not only incidences of illegal use, but also a detriment to the pipe network and water quality, which affects the service to the customer
	Corruption of meter reader	 Corrupt meter readers can create a large impact on the monthly billed consumption Corrupt meter readers will record lower meter readings in exchange for a monetary incentive



[Theft at Hydrant]



Water Loss Control Program

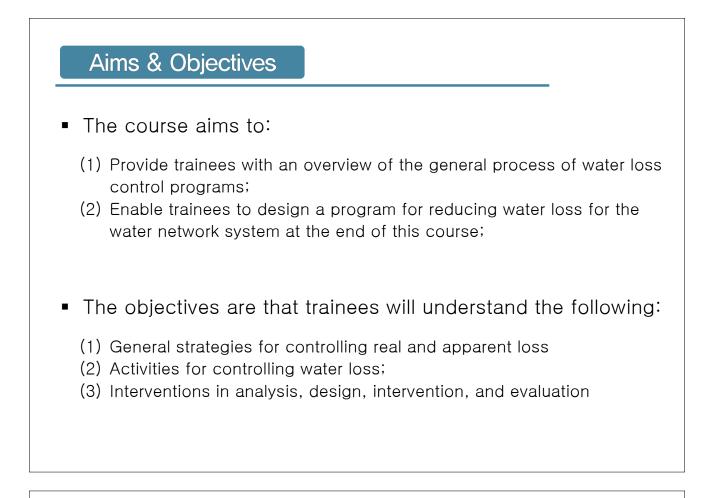
Water Loss Management



Training Course



- 67 -



Contents

- 1. Overview
- 2. General Strategies for Real Loss
- 3. General Strategies for Apparent Loss
- 4. Water Loss Control Programs
- 5. Analysis
- 6. Design
- 7. Intervention
- 8. Evaluation
- 9. Permanent Work

1. Overview

- 1.1 Background
- 1.2 Appropriate Objectives
- **1.3 Holistic Approach**
- 1.4 Flexible and Customized Plans
- 1.5 Understanding the Current Status
- **1.6 Establishing Clear Procedures**
- 1.7 Sustained Commitment: Long-term plans
- 1.8 Systematic Support from the Organization

1.2 Appropriate Program Objectives

- Two main objectives: Water loss reduction activities should be designed to reduce the existing apparent and real losses to economic levels and sustain optimum levels
- Realistic goals: When setting goals, water utilities should be realistic and consider the difficulties of the environment in operation



1.1 Background Technical elements: Water utilities should consider the technical, social, and environmental aspects of water loss control programs Economical elements: Effective water loss control programs can be developed by identifying the types and volumes of water loss, cost of water, and costs of appropriate techniques for reducing the specific components of leakage **Technical** elements Technical Social Environmental Economical Type and volume of leak Cost of water Cost of technique elements [Considerations for Developing Water Loss Control Programs]

1.3 Holistic Approach

- Diagnostic approach: This approach is reinforced by implementing practical and achievable solutions, which can be applied to the development of strategies for water loss management
 - The diagnostic approach firstly identifies problems, then uses available tools to reduce or eliminate such problems
- Limitation of a single task: Water utilities should note that a single project or task cannot resolve NRW reduction
 - Water loss control activities have been grouped into modules based on the types of professionals involved in their execution and the characteristics and level of development of the water distribution network
- Holistic approach: A combination of various methods will constitute the most efficient and economical instrument for water loss reduction, which is dependent on the local situation

1.4 Flexible and Customized Plans

- Flexible and customized plans: A water loss control program should be flexible and tailored to the specific needs and characteristics of a water supply system
- Site-specific plans: Leak management plans should be specific to the water company and supply areas within the company
 - Each company should adopt a strategy for water loss control that suits their reality and improve the quality of service to its customers
- Modified best practices: Many utilities can implement good practices from other utilities with little modification

1.5 Understanding the Current Status

 Assessing water loss: Current water loss should be understood and assessed using a top-down and bottom-up audit before an appropriate strategy for water loss reduction can be developed

- Before deciding which intervention methods are appropriate, a water utility thus should understand the factors that contribute to the real loss in its particular system

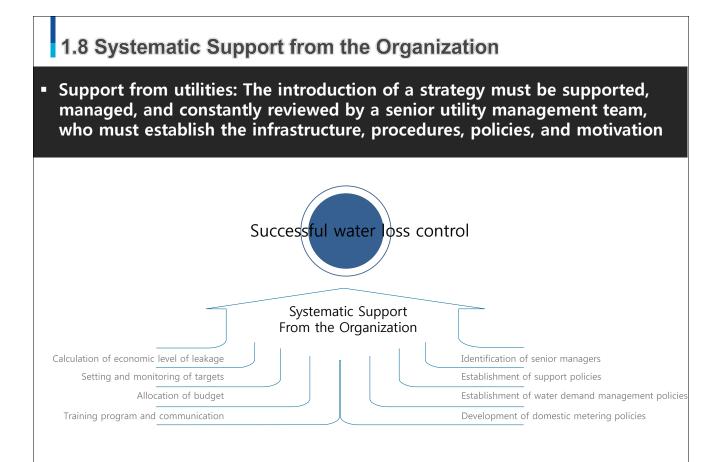
- Identifying the causes: To understand the reasons why, how, and where water is being lost, the managers should perform an appraisal of the physical characteristics of the network and current operational practices
- Understanding the system: Considering a series of conditions and actions to be implemented is imperative to the network that requires prior profound network knowledge, such as characteristics and operation

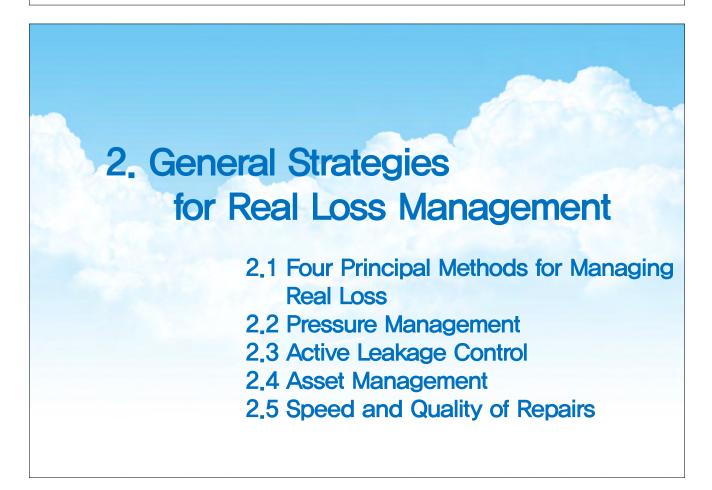
1.6 Establishing Clear Procedures

- Establishing guidelines: Water loss control programs should be designed to provide systematic guidelines, such as a set of procedures, flowcharts, worksheets, and forms
 - Utility managers, engineers, and operations staff can adopt such guidelines for planning and implementing activities
- Role of specialists: Specialists on water loss reduction should determine suitable policies, procedures, and suggestions from the guidelines to enable the staff to initiate a systematic reduction in NRW regardless of the infrastructure, resource, or financial situation experienced by the utility
- General process: It usually includes the following aspects:
 - Understanding the components of water loss and their relative significance
 - Developing action plans to address each cause of the loss
 - Transference of training and skills to ensure the sustainability of good leakage management

1.7 Sustained Commitment: Long-term Plan

- Long-term project: It must be clearly understood that the introduction of a water loss control program must be viewed as a long-term project, which should continue until the economic level of leakage is reached and sustained
 - Immediate savings cannot be expected and water utilities should rather plan for a 5-year or preferably 10-year program
 - The savings will be difficult to achieve, and possibly more difficult to sustain
- Reorganization for sustainability: To achieve permanent results, management procedures associated with a utility's organization, procedures, and human resources should be modified
- Focus on real and apparent losses: Many programs for the reduction of NRW have failed in the long run because they focus on real loss without sufficient attention being paid to apparent loss

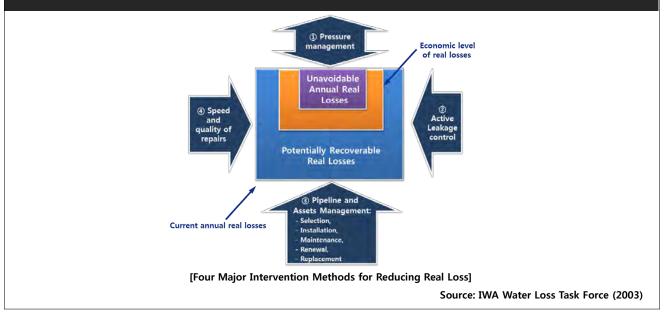




2.1 Four Principal Methods for Managing Real Loss

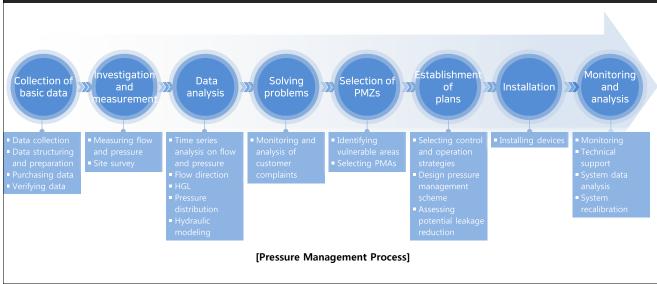
Major intervention methods: In 2003, the IWA Water Loss Task Force defined the four principal intervention methods for reducing real loss

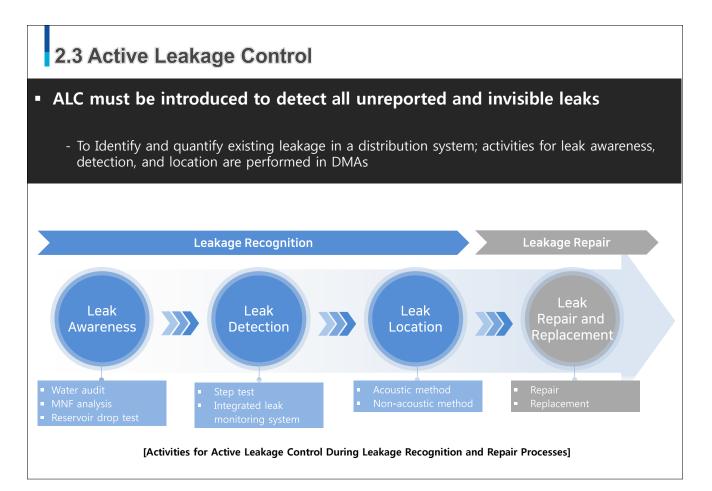
- This chart, which can be found in every book on NRW management, illustrates the entire real loss management issue

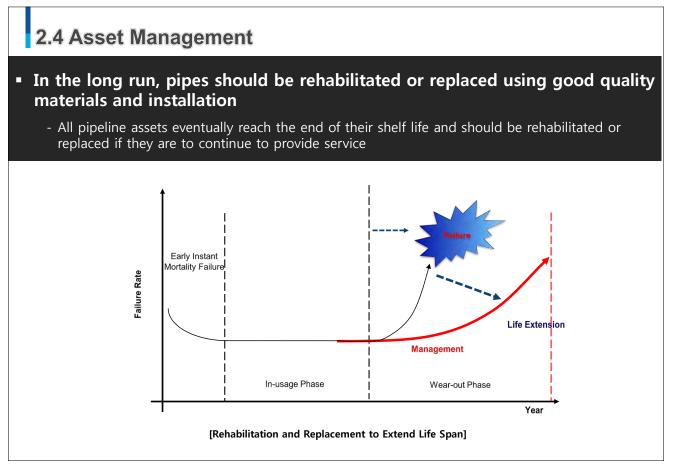


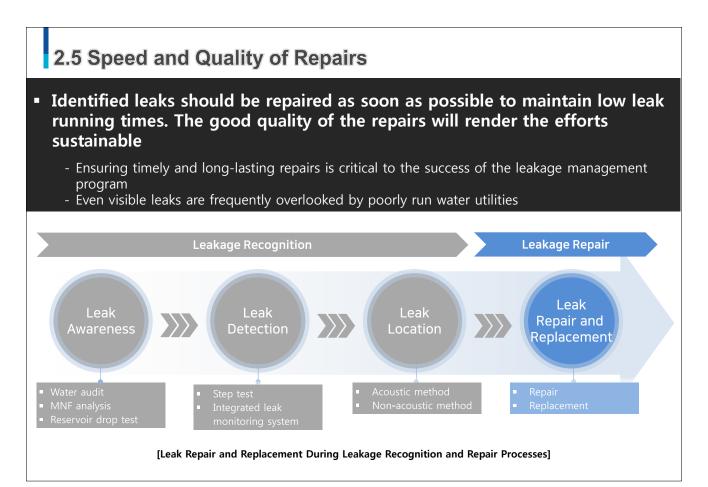
2.2 Pressure Management

- Most effective method: Leakage levels can be improved or worsened solely by changes in the level of operating pressure
- Sole solution: Pressure management is represented by a solely bidirectional arrow because it is the only solution for the reduction of unavoidable annual real loss





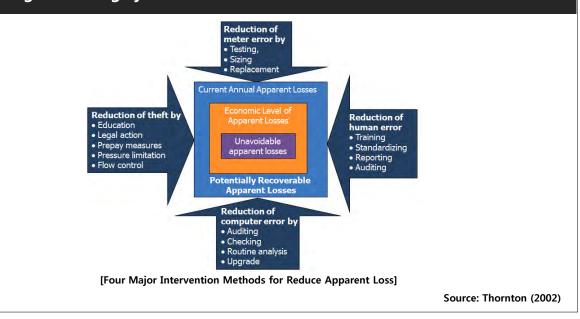






3.1 Four Principal Methods for Managing Apparent Loss

 Major intervention methods: The methodologies for controlling apparent loss require carefully managing water meters, implementing control policies for unauthorized consumption, reducing data handling errors, and optimizing the billing system

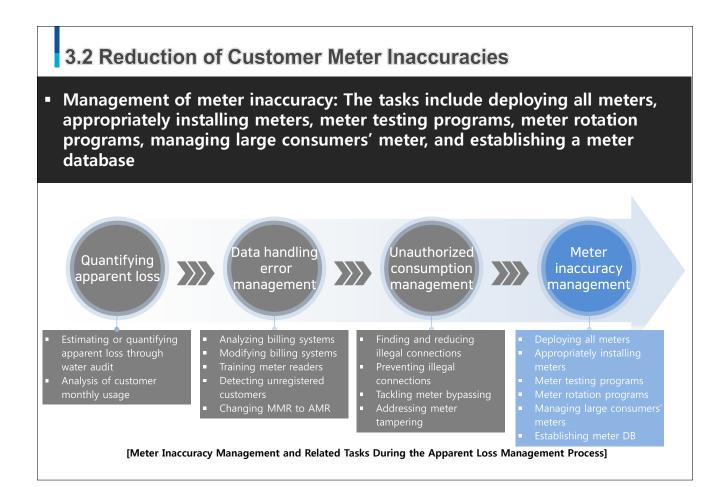


3.1 Four Arrows for Managing Apparent Loss

- Intervention order #1: Reducing data handling error is typically the first step in apparent loss management
 - A general recommendation is that billing system analysis be performed as the initial step because gaps in the process could influence data used in the other steps

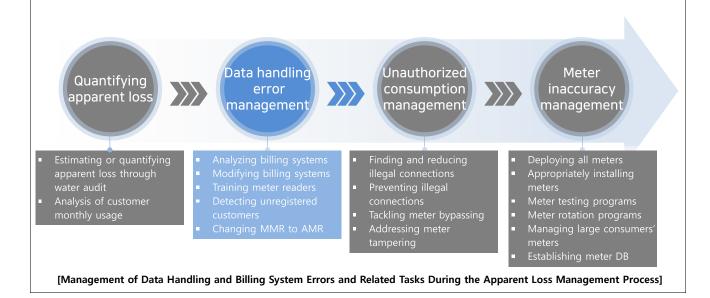
Intervention order #2: Meter replacement is the last option in apparent loss management

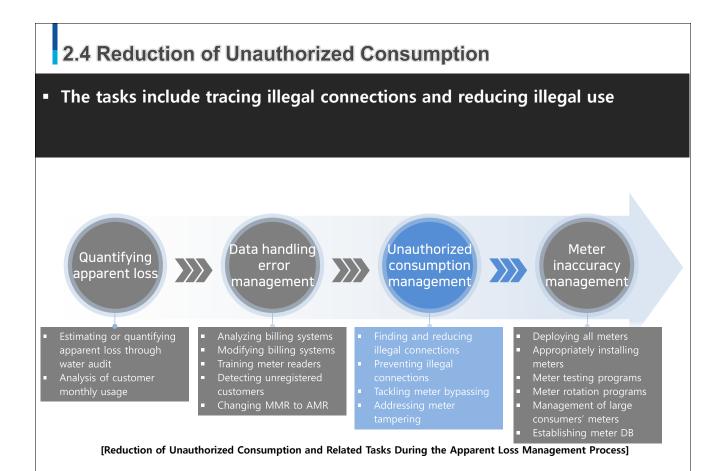
- Many water utilities tend to assume that their system's apparent loss is solely caused by inaccuracy in the customers' meter and that meter replacement of the entire customer population is the best option
- Implementing a comprehensive customer meter changeout is a very expensive and inefficient proposition if the bulk of the apparent loss cases is in fact caused by data errors in the billing system or unauthorized consumption



3.3 Improvement of Data Handling and Billing Systems

 Data handling management: The tasks for the management of data handling errors include analyzing and modifying the billing system, training meter readers, detecting unregistered customers, and shifting from manual meter reading to automatic meter reading

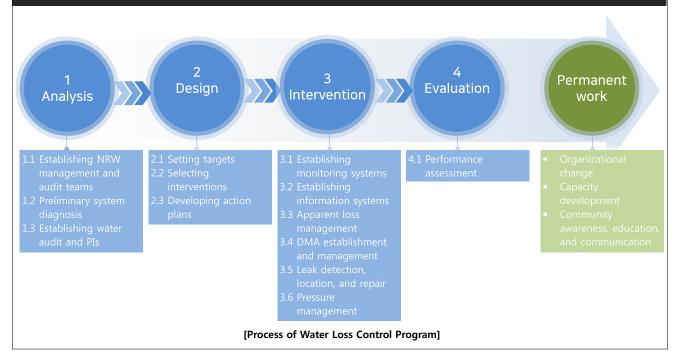






4. Water Loss Control Program

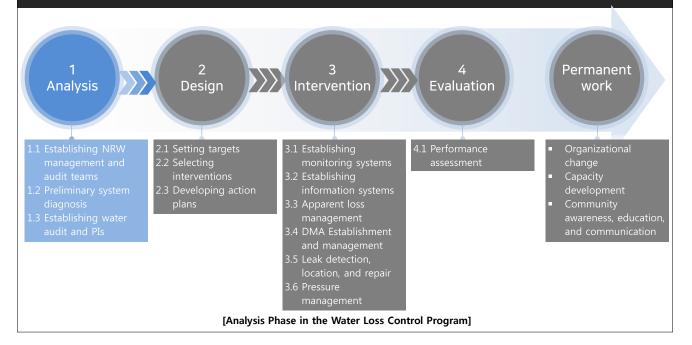
The process is composed of four phases, namely, analysis, design, intervention, and evaluation, with permanent work



5. Analysis Phase 5.1 Analysis Phase 5.2 Establishing NRW Management and Audit Teams 5.3 Preliminary System Diagnosis 5.4 Water Audit and Performance Assessment

5.1 Analysis Phase

The main objectives of the analysis phase are assessing the distribution, metering, and billing operations and determining the volume of water loss and its location



5.2 Establishing NRW Management and Audit Teams

 NRW management team: Water utilities should consider establishing a team to develop a strategy, ensure that all components of NRW are addressed, and verify that the proposed strategy is feasible and practical in terms of workload and budget

 Members: Selecting suitable team members promotes ownership among the various utility departments involved in the implementation of the strategy and facilitates consensus at the senior management level

- The team should comprise members from operational departments, such as production, distribution, customer service, finance, procurement, and human resources

5.2 Establishing NRW Management and Audit Teams

Audit team: The NRW audit team is not responsible for any physical activities to reduce NRW but should be dedicated to assessing all departments involved with NRW activities

- An independent team should be established to audit progress because the process is a utilitywide undertaking

01

TARGETS - The NRW audit team will establish annual

ESTABLISHING ANNUAL

 The NRW audit team will establish annual targets for each department using one or more of the indicators



MANAGING GOVERNANCE

 The head of the NRW audit team will support the chair by providing technical details and progress reports

[Main Tasks of the Audit Team]

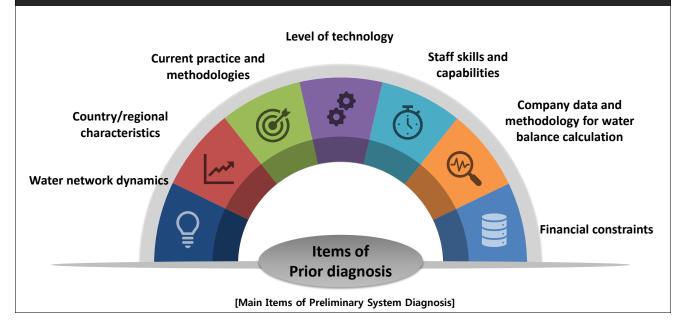
MONITORING MONTHLY PROGRESS

 The team will monitor progress on a monthly basis



5.3 Preliminary System Diagnosis

- Importance of preliminary system diagnosis: Prior diagnosis is necessary to determine a starting point
 - Current water loss must be understood and assessed using a diagnostic approach before an appropriate water loss reduction program can be developed



5.3 Preliminary System Diagnosis

 Sources of data: Items for prior diagnosis can be gathered through existing data, metering, discussion with staff, field visit, and pilot scale test



EXISTING DATA COLLECTION

Collecting existing data from water utility or municipality



METERING

FIELD VISIT

skills

Analyzing the current operating status through metering for flow rate, pressure, and water quality data

Visiting fields to appraise current practices and



DISCUSSION WITH STAFF

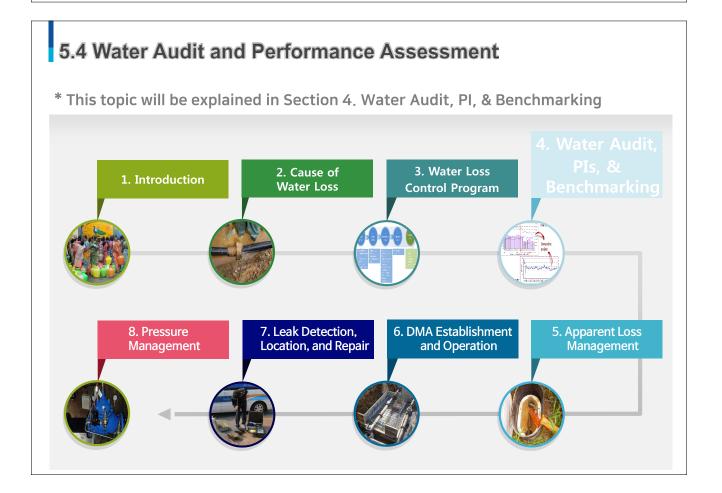
Discussing system features and practice with senior staff and operation staff



PILOT SCALE TEST

Conducting a small-scale pilot test on a suitable pilot area to demonstrate techniques and equipment, gather results, demonstrate benefits, and train staff

[Source of Data]

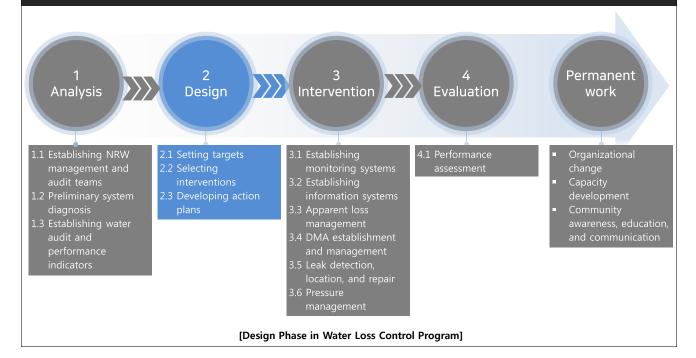


6. Design

- 6.1 Design Phase
- 6.2 Setting Targets
- **6.3 Selecting Interventions**
- 6.4 Developing Action Plans

6.1 Design Phase

 (Objects) The main objects of design phase should be to develop a holistic NRW master plan based on the results of analysis phase and setted targets

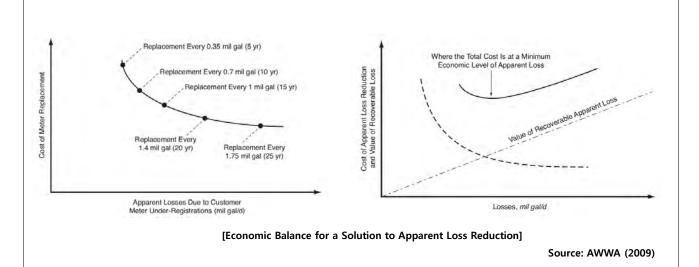


6.1 Setting Targets

- Rough targets: At the early stages of the water loss control program, targets are mainly based on assumptions, which will later be supplemented by accurate information collected throughout the process
 - The strategy development team should firstly set a company-wide target for NRW reduction by considering the other goals or policies of the utility's that may complement or conflict with NRW reduction
- Methods: The economic level of leakage (ELL) or target-setting guidelines can be used in formulating a leakage reduction target
- Factors in consideration: The target level will be a compromise between a series of competing factors, such as available budget, human resources, technical feasibility, time constraints, and even political considerations

6.1 Setting Targets

- Target-setting method #1: By considering ELL, a limit on loss reduction investment is defined, after which exceeding the cost of water production is no longer economically feasible
 - The ELL denotes the level of leakage at which the cost of undertaking additional leakage control activity is equal to the cost of producing water



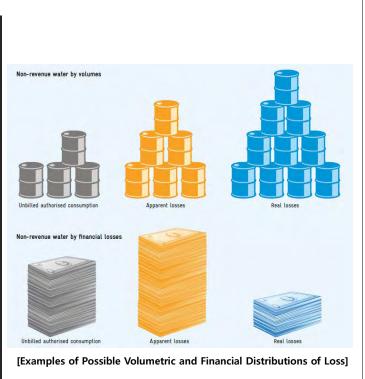
6.1 Setting Targets

- Target-setting method #2: Target-setting guidelines If a water utility has not calculated its ELL, then target-setting guidelines can be used
 - The table, which was developed by the AWWA Water Loss Control Committee, suggests approximate target levels using the ILI and water resources, operational, and financial considerations that utilities typically encounter (Kumkel et al., 2003)

		iminary Leakage Target-setting nation of the system-specific eco	
Target ILI Range	Water Resources Considerations	Operational Considerations	Financial Considerations
1.0-3.0	Available resources are greatly limited and are very difficult and/or environmentally unsound to develop.	Operating with system leakage above this level would require expansion of existing infrastructure and/or additional water resources to meet the demand.	Water resources are costly to develop or purchase. Ability to increase revenues via water rates is greatly limited due to regulation or low ratepayer affordability.
3.0–5.0	Water resources are believed to be sufficient to meet long-term needs, but demand management interventions (leakage management, water conservation) are included in the long-term planning.	Existing water supply infrastructure capability is sufficient to meet long-term demand as long as reasonable leakage management controls are in place.	Water resources can be developed or purchased at reasonable expense. Periodic water rate increases can be feasibly effected and are tolerated by the customer population.
5.0-8.0	Water resources are plentiful, reliable, and easily extracted.	Superior reliability, capacity, and integrity of the water supply infrastructure make it relatively immune to supply shortages.	Cost to purchase or obtain/treat water is low, as are rates charged to customers.
Greater than 8.0	While operational and financial considerations may allow a long-term ILI greater than 8.0, such a level of leakage is not an effective utilization of water as a resource. Setting a target level greater than 8.0—other than as an incremental goal to a smaller long-term target—is discouraged.		
Less than 1.0	In theory, an ILI value less than 1.0 is not possible. If the calculated ILI is just under 1.0, excellent leakage control is indicated. If the water utility is consistently applying comprehensive leakage management controls, this ILI value validates the program's effectiveness. However, if strict leakage management controls are not in place, the low ILI value might be attributed to error in a portion of the water audit data, which is causing the real losses to be understated. If the calculated ILI value is less than 1.0 and only cursory leakage management controls are used, the low ILI value should be considered preliminary until it is validated by field measurements via the bottom-up approach.		
	[Target-setting G	uidelines for Leakage N	Management]

6.2 Selecting Interventions

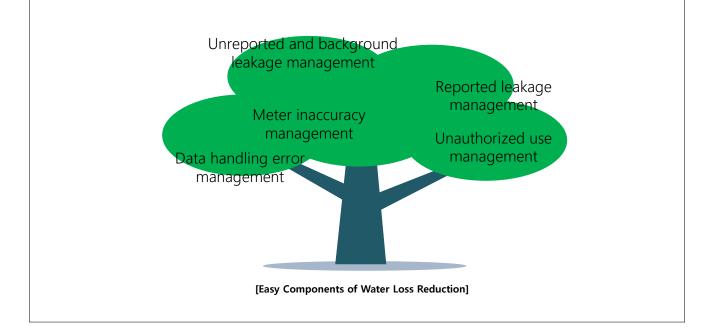
- Prioritizing NRW reduction components: After setting the utility-wide NRW target, utility managers should calculate the proposed volume of water saved by comparing the NRW baseline with the target level
- Comparing components from the financial viewpoint: Water utilities should first focus on the components of NRW, where investments will generate the highest rate of return



Source: GIZ (2011)

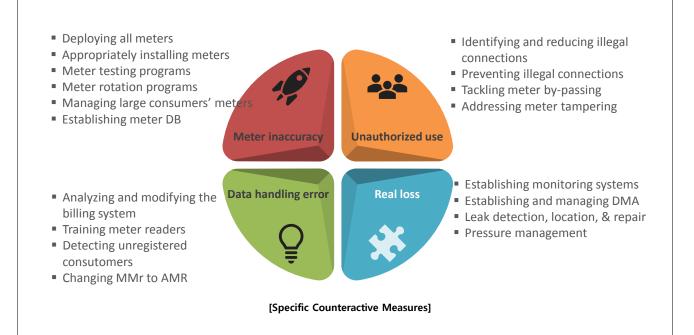
6.2 Selecting Interventions

 Low-hanging fruit (apparent loss): Reducing apparent loss requires a low level of investment with a short payback period but requires sustained management commitment, political will, and community support



6.2 Selecting Interventions

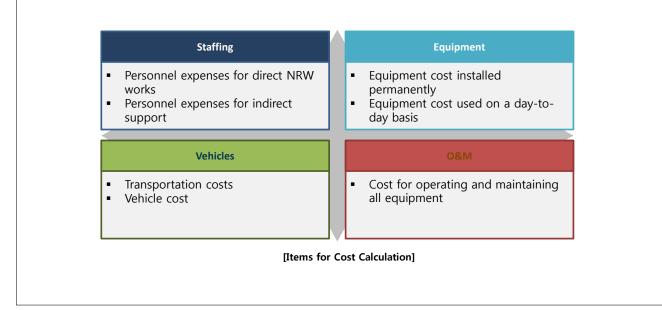
 Counteractive measures: A set of specific counteractive measures is available for each component of NRW



6.2 Selecting Interventions

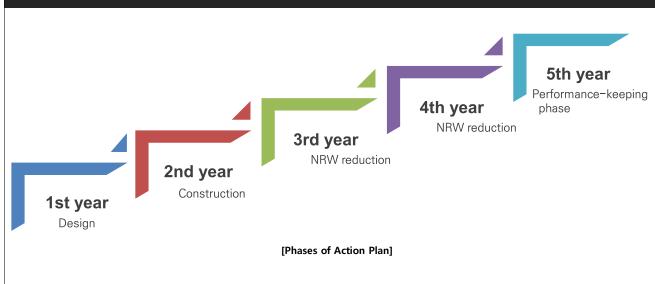
Budget consideration: Many NRW management strategies can last for years. Thus, the overall cost may be relatively substantial

- Many NRW strategies start off at full speed but frequently fail due to budget cuts over time
- Undertaking pilot projects to estimate the cost of the program is useful



6.3 Developing Action Plans

- After understanding the components of water loss and their relative significance, action plans can be developed to address each of the causes of the loss
 - Training and skill transfer are essential components of the program to ensure the sustainability of good leakage management

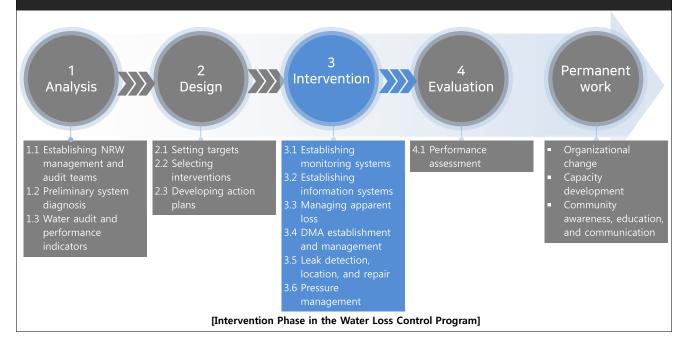




- 7.1 Intervention Phase
- 7.2 Establishing Monitoring Systems
- 7.3 Establishing Information Systems
- 7.4 Managing Apparent Loss
- 7.5 DMA Establishment and Management
- 7.6 Leak Detection, Location, and Repair
- 7.7 Pressure Management

7.1 Intervention Phase

 The main objectives of the intervention phase is selecting interventions and their order on the basis of budget constraints, public benefit, and priority of other scheduled capital improvements



7.2 Establishing Monitoring Systems

Importance of monitoring: Using a reliable system for recording and reporting data is essential for an appropriate understanding and management of the water distribution network

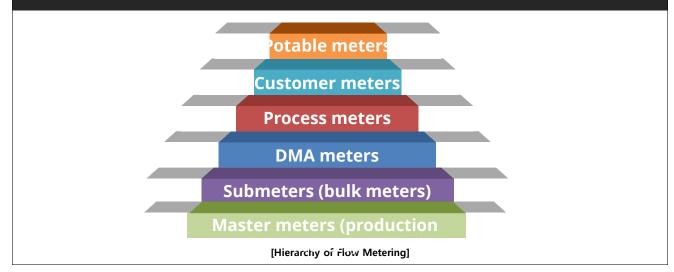


[Integrated Bulk Water Supply Monitoring System]

Courtesy of K-water

7.2 Establishing Monitoring Systems

- Requirements: Suitable selection and field application with appropriate installation are key conditions for the positive performance of meters
- Flow monitoring: The most common scheme for monitoring any water system that aims to locate potential leakage is based on flow meters or water meters located from the source to customers

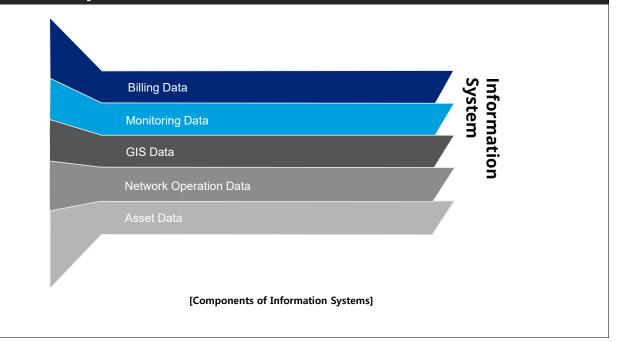


7.3 Establishing Information System

- Importance: Precise information is required for water utilities to enable efficient work
 - This information is derived from data which have to be collected, processed, and interpreted via information systems
- Utilization: Data collection should never become the goal in itself. Instead, water utilities should appreciate the value of collected data
 - Modern water utilities collect and process large amounts of data from different departments and with different departments with various purposes on a daily basis
 - Unfortunately, plenty of collected data remain in the original department without being utilized

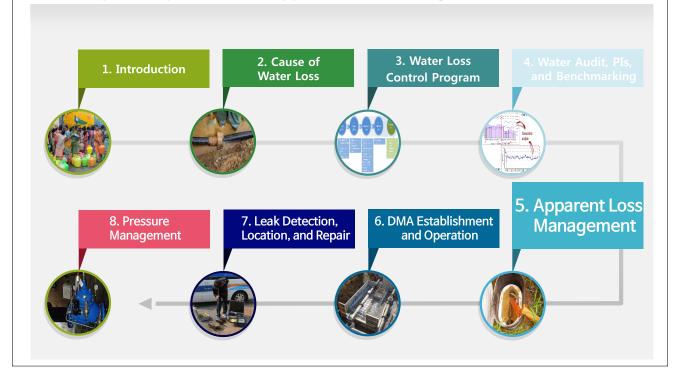
7.3 Establishing Information Systems

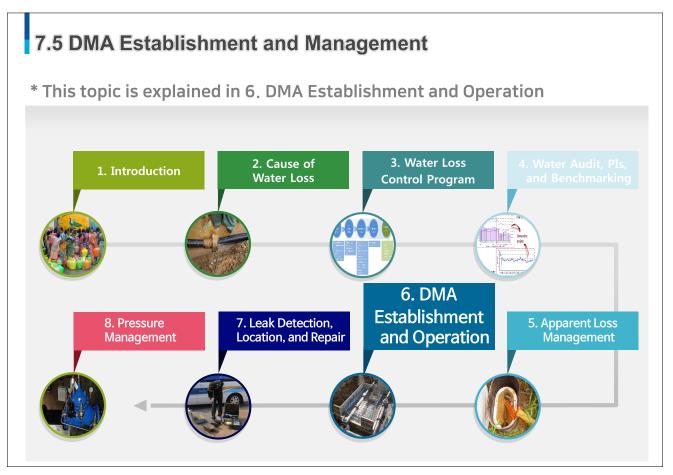
 Components of information systems: Water utilities use several types of information systems with numerous interactions between various information systems

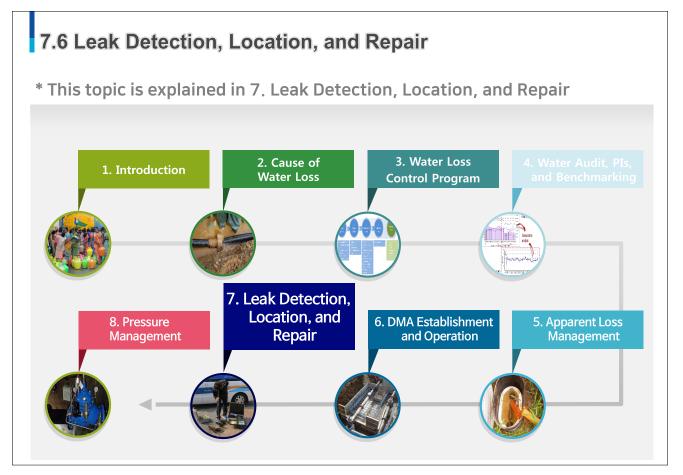


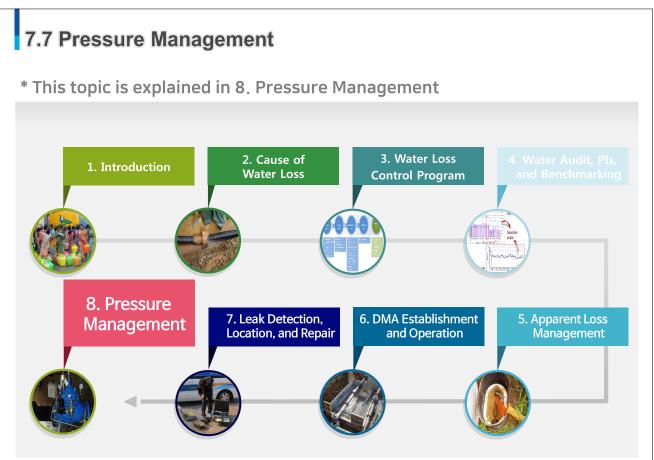
7.4 Apparent Loss Management

* This topic is explained in 5. Apparent Loss Management









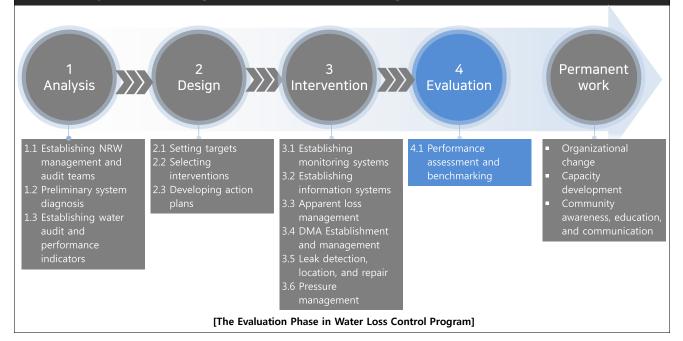
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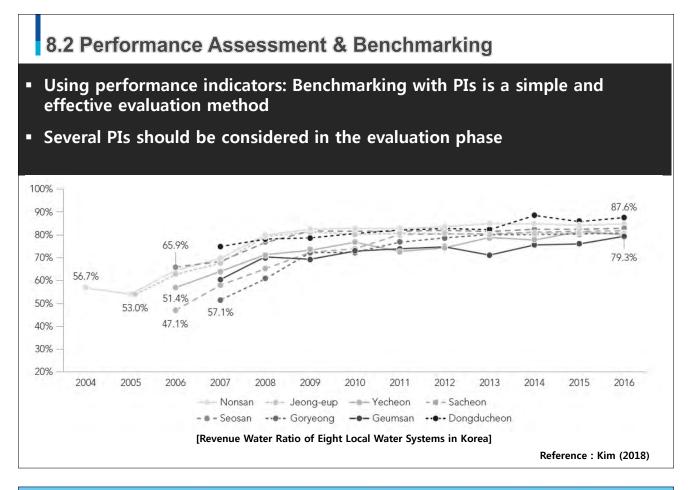
8. Evaluation

- 8.1 Evaluation Phase
- 8.2 Performance Assessment and Benchmarking

8.1 Evaluation Phase

 (Objects) The main objects of evaluation phase are assessing the success of the intervention actions and this evaluation must be carried out on a monthly basis during water loss control porgram







9.1 Permanent Work Objectives: For successful programs and sustainable performance, permanent works should include organizational change, capacity development, and increased community relationship Permanent Intervention Design Evaluation Analysis work 1.1 Establishing NRW 2.1 Setting targets 3.1 Establishing 2.2 Selecting interventions 3.2 Establishing audit teams 2.3 Developing action 1.2 Preliminary system 1.3 Establishing water location, and repair 3.6 Pressure [Permanent Works in the Water Loss Control Program]

9.2 Organizational Changes

- Required organizational changes: Effectively addressing NRW requires a combined effort from the management and staff throughout the utility
- Divided structure: One of the most serious problems faced by many water utilities in developing countries with regard to water loss control is the divide between the technical and financial departments within the utility
- Best practice: Indeed, the fact that when utilities change from a relatively
 passive attitude toward the problem of water loss to a proactive approach in
 conjunction with the emergence of positive results from the new strategy
 tends to promote an increase in motivation that initially gives rise to a new
 dynamism across the utility structure

9.3 Capacity Development

- Importance: NRW reduction is not only a technical issue. Implementing a successful NRW program and achieving strong results require committed management and trained staff that continuously work to retain low levels of NRW
 - To adapt and reap the full benefits from an NRW management system, the water utility must implement a comprehensive training and technology transfer program to secure continued positive results
- Definition: Capacity development is the process of strengthening the abilities of individuals, organizations, companies, and societies to effectively and efficiently utilize resources and realize goals on a sustainable basis
- Agreement: Trainers and trainees should reach a consensus on a carefully designed strategy for capacity development, which includes a comprehensive training concept, because conditions vary across countries

9.4 Community Awareness, Education, and Communication

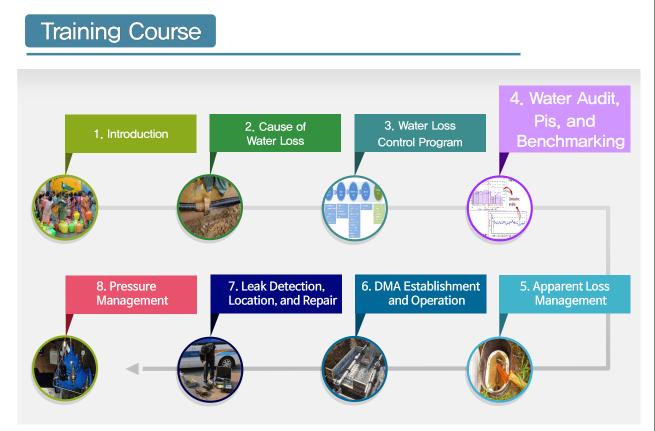
- Importance: Water loss is not only an engineering problem but also a reflection of the sociocultural situation that requires changes in community behavior and attitude toward water usage.
 - For many years, community awareness and education have been considered the ugly, useless step-child of water demand management.
- Critical factor to success: Frequently, well-designed and implemented technical interventions fail miserably because the community being served is excluded from the overall process. As a result, the community does not *buy-into* the project.
- Difficulty in monitoring: Unfortunately, activities geared toward community awareness and education are difficult to monitor with regard to cost compared to savings achieved. Both aspects are impossible to quantify.



Water audit & Performance Indicators

Water Loss Management





Aims and Objectives

- The course aims to:
 - Provide a background on the water audit process, Pls, and benchmarking methods;
 - (2) Enable trainees to perform a water audit of and evaluate the system
- The objectives are that trainees will understand:
 - (1) Top-down and bottom-up approaches in water balance calculation;
 - (2) Performance indicators

Contents

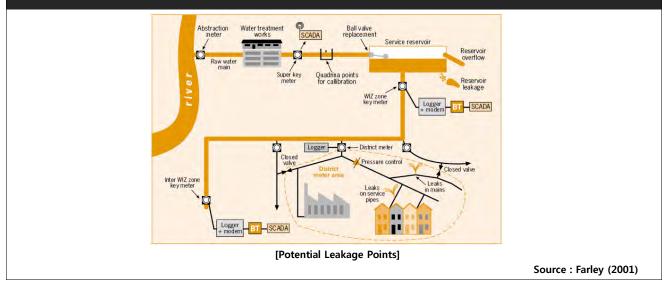
- 1. Overview
- 2. Introduction
- 3. Top-down Approach
- 4. Bottom-up Approach: Minimum Night Flow Analysis
- 5. Performance Indicators and Benchmarking
- 6. Tools and S/W
- 7. Conclusions

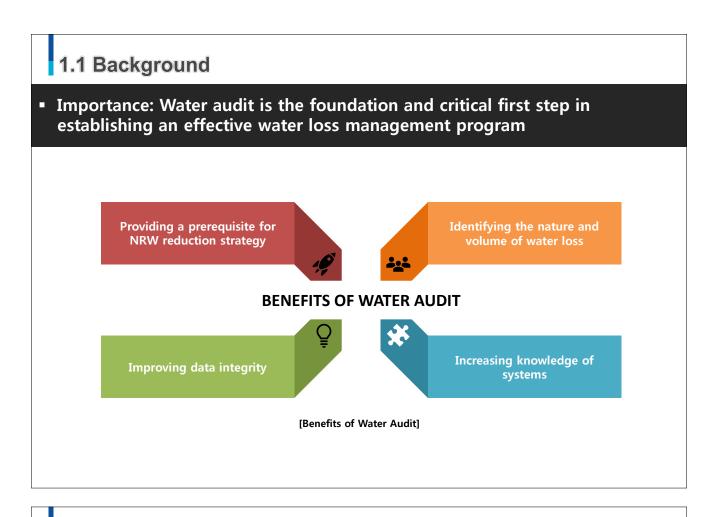
1.1 Background 1.2 Water Audit Approach 1.3 Standardized Definitions of Terminologies and Methods 1.4 Issues in Application

1.1 Background

Definition: Water audit determines the amount of water loss from a distribution system

- Water audit quantifies the total water loss and leakage in a network
- In effect, it is a water balance calculation, where the amount of water in distribution is compared with the sum of the components of water consumed or used
- Water balance is called water audit in the USA



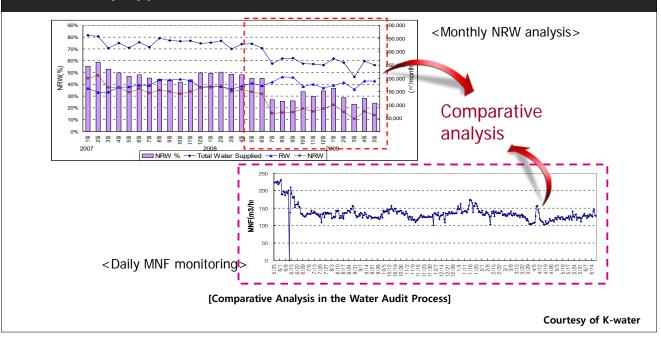


1.1 Background

- From guess to science: Thirty years ago, NRW management was based on guesswork than on precise science
 - However, this tendency has changed dramatically for many industrialized countries, which was kick-started by the regulatory pressure on UK water companies to cut leakage
- Amount, not location: NRW assessment basically involves quantifying loss in a particular system without considering the location of the loss
- Tools: Designing a worksheet or spreadsheet to perform water balance calculations is recommended
 - Commercial software is available to support collection of necessary data and performing calculations
 - A number of techniques for calculating water balance exist and should be combined to achieve reliable results

1.1 Background

 Comparative analysis: Utility managers must, therefore, verify the result using component analysis (top-down approach) or physical loss assessment (bottom-up approach)



- 1.1 Background
- GIGO: Noting that the accuracy of water loss volume is dependent on the accuracy and quality of data used in the calculation is extremely important
 - Reliable metering of all water volumes entering and exiting the supply system is a fundamental requirement
 - Data validation also play a key role in assessing water loss volume
- Wrong Information: Reported low levels of NRW whether due to deliberate misinformation or, more likely, lack of accurate information, will be unable to help the water utility to reduce costs or increase revenue

- Instead, it will mask the real problems affecting the operating efficiency of the water utility

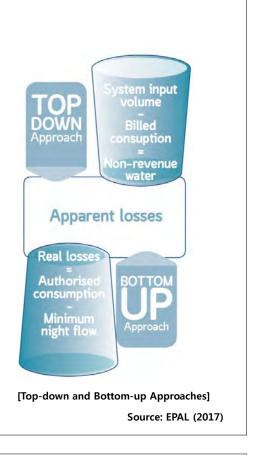
1.2 Water Audit Approach

Water balance calculation can be classified into top-down and bottom-up approaches

- First, a top-down water balance should be prepared using the IWA methodology and continually refined as data become available to enable a bottom-approach

Objectives of each approach

- Top-down: estimation of crude water balance
- Bottom-up: complementary analysis for top-down analysis from the water balance point of view and analysis in DMA-scale and prioritization between DMAs for active leakage detection and leak awareness



1.3 Standardized Definitions of Terminologies and Methods

- Precondition for BM: The precondition for calculating internationally comparable PIs is a standardized terminology and water accounting methodology
 - Until the early 1990s, no reliable and standardized methods were available for accounting for water loss
 - Leakage management performance was measured in terms of unaccounted-for water

Standardization: In 2000, the International Water Association (IWA) developed standard terminologies and methods to assist water systems in tracking water loss and performing water audit

- As a part of an initiative started in the late 1990s, the IWA established the Water Loss Task Force (WLTF) to examine international best practices and develop PIs related to water loss
- The IWA developed standard terminologies and methods to assist water systems in tracking water loss and in performing water audit (Lambert & Hirner, 2000)
- The IWA water audit methodology is based on the water balance table, which specifies different types of water consumption and loss

1.4 Issues in Application

(1) Not calculating a water balance

- (Not) performing water audit: Many water utilities are not used to producing a water balance
 - Therefore, the split between physical and commercial losses is rarely known
 - The scenario renders the calculation of meaningful NRW PIs nearly impossible
- Valuable, even if wrong: Establish a water balance is always worth the effort, even if main elements are based on estimates
 - By so doing, producing a catalogue of required actions to improve the accuracy of water balance becomes possible

1.4 Issues in Application

(2) Lack of data

- Lack of data: Frequently, even the most basic information, such as system input volume, average pressure, supply time, length of mains, and number of service connections, is initially unavailable
 - The process of calculating each component and PI of water balance will reveal such deficiencies
 - Utility management should then take corrective action to close these data gaps and improve data quality
- Absence of monitoring system: Most water utilities lack adequate monitoring systems for assessing water loss, and many countries lack national reporting systems that collect and consolidate information on water utility performance
 - The result is that data on water audit are not readily available
 - Even available, data may not be always reliable because several poorly performing utilities are known to practice window dressing to conceal the extent of their inefficiency

1.4 Issues in Application GIGO: Using incomplete or inaccurate data for water balance calculation will not produce useful results – garbage in, garbage out Required data: An extremely important aspect to note is that the accuracy of water loss volume is dependent on the accuracy and quality of data used for calculation The top-down method requires that bulk and customer water meters are installed and read for the period of at least 1 year The accuracy of installed bulk meter should be determined via field tests, e.g. through a volumetric test, portable insertion, or clamp-on flow meters 1.4 Issues in Application

(3) Manipulating NRW

Fabrication: The amount of publicly available data water audit is limited. Even worse, data are frequently wrong and misleading

- In countries where the legal situation requires water utilities to maintain the level of NRW below a certain level, a strong tendency occurs to report lower-than-actual numbers to meet legal or regulatory requirements
- For example, this scheme was common in the former Soviet Union, where utilities were required to maintain NRW below 18%. All utilities reported low levels, despite actual levels often exceeding 50%

1.4 Issues in Application

(4) How can the water utility estimate unmetered components?

- Accurate estimates: In the absence of actual metering, effort should be made to reach a precise assessment of each component of water volume and use to determine realistic quantities for water balance
 - Methods used to estimate any unmetered components of the water balance should be recorded and defined
- AL in developing or developed countries: Although AL is a major concern in low- and mid-income countries, it lacks recognition in input data for the majority of water audit analyses
 - Providing extensive detailed input for AL results in accurate results

2. Top-down Approach

- 2.1 Top-down Approach
- 2.2 IWA 'Best Practices' for Water Balance
- 2.3 Procedure
- 2.4 Defining Key Parameters Before Water Audit
- 2.5 Estimates
- 2.6 Quantified Values
- 2.7 Uncertainty Analysis

2.1 Top-down Approach

- Starting point of audit: The top-down approach is the recommended starting point for water utilities compiling an initial water audit
- Paper exercise: The top-down approach initiates the process with all available information, usually being undertaken on paper or as a desktop exercise, in which no field work is but a number of estimates are made

- This approach begins with a macro-analysis of losses that assesses the need for intervention over the whole network

 Interval: Each water utility should perform a top-down water balance on an annual basis

2.1 Top-down Approach



Preliminary assessment of water loss

With relatively modest effort, the top-down method can provide a good preliminary assessment of water loss status and insight into the quality of available data on water supply

Identifying components for validation

The top-down audit identifies components that require further validation

Easy PI calculation

After calculating the IWA standard water balance, best practice NRW PIs can be calculated for target monitoring and leakage benchmarking

Errors and uncertainty

The procedure is subject to errors and uncertainty

Retrospective approach

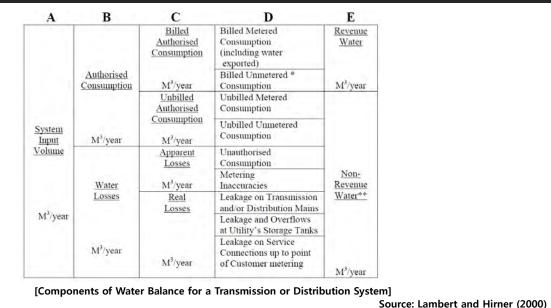
The utility should consider that the annual water balance is a retrospective approach. In other words, it cannot provide an early warning system for identifying new leaks and bursts

[Pros and Cons of Top-down Approach]

2.2 IWA "Best Practice" for Water Balance

Best Practice: The IWA developed the top-down annual water balance as a best practice method

- The top-down method requires that bulk and customer water meters are installed and read for a period of at least one year



2.2 IWA "Best Practice" for Water Balance

RL and AL: Water losses consist of real and apparent losses

- Real loss: burst pipes, leaking joints, fittings, service pipes, and connections
- Apparent loss: illegal connections, under-registration of customer meters, inaccurate meters, stopped meters, vandalized meters, by-passed meters, billing errors, inadequate meter reading policies, and bribery and corruption of meter readers
- Administrative NRW: Unbilled authorized consumption and apparent loss are referred as administrative NRW
- Terminology: Unbilled, unmetered authorized consumption is any kind of authorized consumption this is neither billed nor metered
 - This component typically includes items, such as fire fighting, flushing of mains and sewers, street cleaning, and frost protection
 - In many countries, water is provided for free to certain consumer groups (i.e., religious institutions)
- Room for manipulation: Unbilled, unmetered authorized consumption, which traditionally includes water used by the utility for operational purposes, is frequently seriously overestimated
 - Oftentimes, the underlying cause is simplification (e.g., using a percentage of the total system input) or deliberate overestimation to reduce the amount of NRW

2.3 Procedure

(1) IWA (2000)

• Steps for calculating NRW and water loss

A	В	С	D	E	
		Billed Authorised Consumption	Billed Metered Consumption (including water exported)	Revenue Water	Steps for Calculating Non-Revenue Water and Water Losses Step 1: Define System Input Volume and enter in Col. A Step 2: Define Billed Metered Consumption and Billed Unmetered Consumption in Col. D; enter total in Billed
	Authorised Consumption	M ³ /year	Billed Unmetered * Consumption	M ³ /year	Authorised Consumption (Col. C) and Revenue Water (Col. E)
		Unbilled Authorised	Unbilled Metered Consumption		Step 3: Calculate the volume of Non-Revenue Water (Col. E) as System Input Volume (Col. A) minus Revenue Water (Col. E)
System Input	M ³ /year	Consumption M ³ /year	Unbilled Unmetered Consumption		Step 4: Define Unbilled Metered Consumption and Unbilled Unmetered Consumption in Col. D; transfer total to Unbilled Authorised Consumption in Col. C
Volume		Apparent Losses	Unauthorised Consumption		Step 5: Add volumes of Billed Authorised Consumption and Unbilled Authorised Consumption in Col. C; enter sum as Authorised Consumption (top of Col.B)
	Water	M³/year	Metering Inaccuracies	<u>Non-</u> Revenue	Step 6: Calculate Water Losses (Col. B) as the difference between System Input Volume (Col.A) and Authorised Consumption (Col. B)
	Losses	Real Losses	Leakage on Transmission and/or Distribution Mains	Water**	Step 7: Assess components of Unauthorised Consumption and Metering Inaccuracies (Col. D) by best means available add these and enter sum in Apparent Losses (Col. C)
M ³ /year			Leakage and Overflows at Utility's Storage Tanks		Step 8: Calculate Real Losses (Col. C) as Water Losses (Col. B) minus Apparent Losses (Col. C)
	M ³ /year	M ³ /year	Leakage on Service Connections up to point of Customer metering	M ³ /year	Step 9: Assess components of real losses (Col. D) by best means available (night flow analysis, burst frequency/flow rate/duration calculations, modelling etc), add these and cross-check with volume of <i>Real Losses</i> in Col. C which was derived from Step 8

[Components of Water Balance for a Transmission or Distribution System]

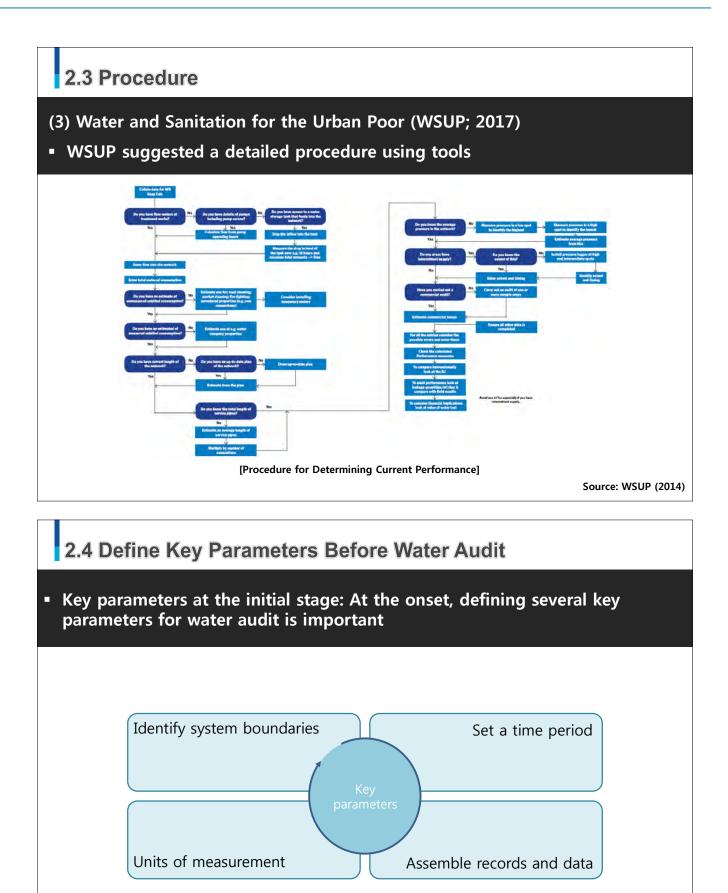
Source: Lambert and Hirner (2000)

2.3 Procedure

(2) AWWA (2006)The procedure is provided in detail in the AWWA M36 manual

- AWWA suggests similar procedure with 10 tasks

Step	Task
1	- Collect distribution system description information
2	- Measure Water supplied to the distribution
3	- Quantify billed authorized consumption
4	- Calculate Nonrevenue water
5	- Quantify unbilled authorized consumption
6	- Quantify water loss
7	- Quantify apparent losses
8	- Quantify real losses
9	- Assign cost of apparent and real losses
10	- Calculate the performance indicators



2.5 Estimates

- Based on measurement: Water balance should be based preferably on the actual measurement of flow rates and volumes
- Last resort (estimation): However, in the absence of a reliable duly verified measurement, every effort should be made to evaluate as accurately as possible the volume and consumption of each component and realistically estimate each component of water balance

2.5 Estimates

 Estimated components: In the top-down water audit, unbilled authorized consumption and apparent loss components can be estimated

- To determine inaccuracies in customer meters, a representative sample is tested in the laboratory at different flow rates representative of the field condition
- Data handling and billing errors are estimated by investigating historical billing records and trends
- Estimating the amount of unauthorized use is challenging. Therefore, it is commonly assumed arbitrarily

	Parameter		Suggested Default				
Unbilled U	Inmetered Authorised	d Consumption	0.5%	of Billed Authorised Metered Consumption			
L	Inauthorised Consum	ption	0.2%	(excluding Water Exported)			
C	Direct pressure	Residential	2.0%	of metered residential consumption			
Customer	systems	Non-residential	2.0%	of metered non-residential consumption			
Meter Inaccuracies	Storage tanks supplied by gravity	Residential and Non-Residential		 > 5% of metered consumption, influenced / factors. Assess on country by country basis. 			

Note: default volumes to be calculated as simple % of metered consumption

[Suggested Estimates for Water Audit]

Source: Lambert et al. (2014)

2.5 Estimates

 Selection between estimated and quantified values: To quickly quantify this category, the default value can be used instead of quantifying numerous minor wastes, uses, or losses

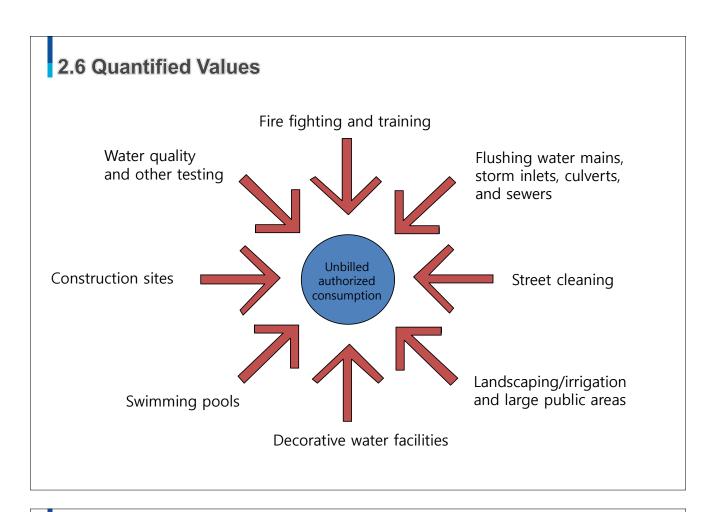
- Quantifying work can be time-consuming. The auditor should use good judgment to determine whether the extra effort to analyze many undocumented occurrences of consumption is likely to lead to a consumption level that exceeds the default value
- A recommendation is that the default value should be applied unless the auditor has documented evidence

Preference	Details
Estimate	 In general, the auditor's time will be better saved if dedicated to quantifying and controlling real and apparent losses In most cases, the extra effort to document these consumptions is not worthwhile
Quantified value	 Under certain conditions, such as severe drought, the publicly visible use of water for flushing or other operations could generate negative public perceptions for water utilities. In such cases, auditing should review all instances of unbilled authorized consumption and ensure efficient management If the auditor feels that the consumption is notably larger than the default value, then detailed estimates of these components can be obtained

2.6 Quantified Values

(1) Quantifying unbilled authorized consumption

- No measurement and flat fee: Unbilled authorized consumption is rarely metered. It can be directly billed, although revenue is frequently recovered via flat fees paid by the fire department or other users
- No policies and procedures: Unfortunately, many water utilities lack clearly written policies that include procedures for safely supplying such an unbilled water consumption
 - Similarly, good accounting is lacking for the types and volumes of such consumption occurring throughout the year
 - A recommendation is that the auditing process should review utility policies and practices and improve as needed to ensure that such water consumption is safe or appropriately utilized and can be accounted for in terms of the extent of its practicality



2.6 Quantified Values

(2) Quantifying apparent losses

- Difficulty: Quantifying apparent loss in a distribution network is a difficult task, which has to be based mostly on assumption and estimates
- No meter, no meter inaccuracy: For water utilities with unmetered customer consumption, no amount of apparent loss is caused by customer meter inaccuracy. This component does not apply
- No general method: Accurate methods for calculate apparent loss values are lacking. However, recommendations are provided for estimating the indicators of this type of loss

2.6 Quantified Values

Quantification of three components: These assumptions should be drawn separately for each of the three main sources of apparent loss as follows.

- Although assumed volumes of apparent loss are subject to errors and thus approximate to a certain degree, this step is required to determine water balance

Main source	Details
Assessing meter inaccuracies	 Apparent loss due to meter inaccuracies can be estimated by selecting a representative group of domestic meters while taking into account the different meter types, brands, sizes, and age groups and verifying them using a test bench Large bulk meters can be tested on-site and in operation through calibrated portable flow meters Average meter inaccuracies (percentage of over- or under-registration) for each group of meters can then be applied to the entire meter park
Assessing data handling errors	 Errors during data handling can be detected and quantified through standard database software Meter reading errors can be reduced through a continued training of meter readers and by introducing standardized working routines
Assessing unauthorized consumption	 The water utility should estimate the number of illegal connections to quantify unauthorized used of water, e.g., by conducting a house-to-house survey in a pilot zone The estimated number of illegal connections should be multiplied by the typical household size and per capita water consumption Illegal connections accidentally discovered by water utility workers should thus be registered The number of broken, by-passed, or manipulated meters should be taken into account based on information from meter readers or other utility personnel

2.6 Quantified Values

 Meter inaccuracies: From the random sampling test, the weighting factors for flow rates and mean registration can be derived. These values can be used to quantify apparent loss as a result of meter inaccuracies

- The test should be classified into two categories, namely, small-sized meter for residential use and large-sized meter for industrial, commercial, or agricultural use
- Weighting factors for flow rates:
- Mean registration: accuracy

Percent of Time	Range, gpm	Average, gpm	% Volume [†]						
15	Low 0.50-1.0	0.75	2.0						
70	Medium 1–10	5.00	63.8						
15	High 10-15	12.50	34.2	-		11.1			
Meters," Journal AWWA, 6:2 Percent volume refers to the	96 (1982). proportion of water consume	ampling Technique for Contro d at the specified flow rate, as of the total water consumed or	compared to the total volume	Percent Volume* (%V)	Total Sales Volume [†] (Vt), mil gal	Volume at Flow Rate (Vf) (%V × Vt), mil gal	Meter Registration (R) [‡] , %	Meter Error (ME) ME = Vf/(0.01R) - Vf, mil gal	Meter Error (ME), mil ga
approximately 0.5–1.0 gpm Instead of using the p	ercentage of volumes shown	here, the utility may compute	its own percentage volume	2.0	2,3188	46.38	88.8	[(46.38/0.888) - 46.38]	5.85
data. Using special dual-me water meters.	eter yokes and recording mete	ers, the utility can determine t	he actual flow rates for their	63.8	2,318.8	1,479.39	95.0	[(1,479.39/0.95) - 1,479.39]	77.86
				34.2	2,318,8	793.03	94.0	[(793.03/0.94) - 793.03]	50.62
						Total residential	meter error (Lin	e 18 of Figure 2-4)	134.33
Test Flow	Rates, gpm	Mean Regi	stration, %						
Low	(0.25)	88	1.8						
Madin	m (2.0)	95	i,0	-					
meun	(15.0)	0.	.0						

Source: AWWA (2009)

		adin	incu	Valu	63										
		Flow Rates			% Volun	ne Deliv	ered								
		Low				10									
		Medium				65			_						
		High				25									
										Percent	Total Sales	Volume at Flow Rate (Vf)	Meter	Meter Error (ME) ME =	
									_	Volume* (%V)	Volume [†] (Vt), mil gal	(%V × Vt), mil gal	Registration (R) [‡] , percent	Vf/(0.01R) – Vf, mil gal	Meter Error (ME), mil ga
			3.5		2	Vari (design	n Registrati ious Flow R nated as per	ates: cent of	+	Volume*			Registration		
	Size.	Meter Type	Date of	Manufacturer	Test Date	Vari (design	ious Flow Ra nated as per registration	ates: cent of	+	Volume* (%V) 10	mil gal 939.2	mil gal 93.92	Registration (R) [‡] , percent 90.0	mil gal [(93.92/0.90) - 93.92]	(ME), mil ga 10,43
Jumber	Size. in. 3	Meter Type Turbine	Date of Installation June 1991	Manufacturer Sensus	Test Date Oct 2006	Vari (design	ious Flow Ra nated as per	ates: cent of	+	Volume* (%V) 10	mil gal 939.2	mil gal 93.92	Registration (R) [‡] , percent 90.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01)	(ME), mil ga 10.43
Vumber VYZ001	in.		Installation			Var (design Low	ious Flow Ra nated as per registration Medium	ates: cent of High	+	Volume* (%V) 10 65	mil gal 939.2 939.2 939.2 939.2	mil gal 93.92 610.48 234.80	Registration (R) [‡] , percent 90.0 96.54 101.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01) - 234.80]	(ME), mil ga 10,43 21.86 -2.32
Number XYZ001 X00ZAA	in. 3	Turbine	Installation June 1991	Sensus	Oct 2006	Var (design Low 89	ious Flow Ranated as per registration Medium 93	ates: cent of High 100	+	Volume* (%V) 10 65	mil gal 939.2 939.2 939.2 939.2	mil gal 93.92 610.48 234.80	Registration (R) [‡] , percent 90.0 96.54 101.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01)	(ME), mil ga 10.43 21.86
Vumber XYZ001 X00ZAA NB123	in. 3 3	Turbine Turbine	Installation June 1991 June 1993	Sensus Sensus	Oct 2006 Oct 2006	Var (design Low 89 70	ious Flow Ra nated as per registration Medium 93 95.2	Ates: cent of High 100 98	+	Volume* (%V) 10 65	mil gal 939.2 939.2 939.2 939.2	mil gal 93.92 610.48 234.80	Registration (R) [‡] , percent 90.0 96.54 101.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01) - 234.80]	(ME), mil ga 10,43 21.86 -2.32
Number KYZ001 K00ZAA NB123 NB456	in. 3 3 4	Turbine Turbine Displace	Installation June 1991 June 1993 July 1980	Sensus Sensus Sparling Sparling Hersey	Oct 2006 Oct 2006 Oct 2006 Oct 2006 Oct 2006	Vari (design 89 70 95 98 98	ious Flow R: nated as per registration 93 95.2 99 96.5 99	attes: cent of High 100 98 102 102 103	+	Volume* (%V) 10 65	mil gal 939.2 939.2 939.2 939.2	mil gal 93.92 610.48 234.80	Registration (R) [‡] , percent 90.0 96.54 101.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01) - 234.80]	(ME), mil ga 10,43 21.86 -2.32
Meter ID Number XYZ001 X00ZAA NB123 NB456 AA002	in. 3 4 6	Turbine Turbine Displace Compound	Installation June 1991 June 1993 July 1980 Sept 1977 May 1966	Sensus Sensus Sparling Sparling	Oct 2006 Oct 2006 Oct 2006 Oct 2006 Oct 2006 egistrations	Var (design Low 89 70 95 98	ious Flow R: nated as per registration 93 95.2 99 96.5	Ates: cent of High 100 98 102 102	+	Volume* (%V) 10 65	mil gal 939.2 939.2 939.2 939.2	mil gal 93.92 610.48 234.80	Registration (R) [‡] , percent 90.0 96.54 101.0	mil gal [(93.92/0.90) - 93.92] [(610.48/0.9654) - 610.48] [(234.80/1.01) - 234.80]	(ME), mil ga 10,43 21.86 -2.32

Source: AWWA (2009)

2.7 Uncertainty Analysis

- Inherent error: Water balance is based not only on measurements but also on estimates of water production, consumption, and losses
 - Hence, the resulting volumes of NRW and real and apparent losses will be subject to a greater or lesser degree of error

 Effect of error: Errors in determining real and apparent loss volumes will influence the economic analysis of options and may lead to an inappropriate strategy for water loss reduction

- Therefore, an important aspect of this scheme is that all measured data and necessary estimates should be as close to the real conditions as possible, such that valid and useful results can be achieved
- Importance of uncertainty analysis: Introducing the uncertainties of the water balance components is a significant achievement in raising awareness of the limitations of water balance and practical means to improve them
 - Uncertainty analysis points out that input data should be verified to minimize the uncertainty of an interesting output

2.7 Uncertainty Analysis

- Benefit #1: Using uncertainty analysis to improve output is not questionable and strongly recommended (Alegre et al., 2016; AWWA, 2016; Lambert et al., 2014)
- Benefit #2 Uncertainty analysis can improve the output of water loss assessment methods, although it does not demonstrate the accuracy or validity of methods (Al-Washali et al., 2020)
- Accuracy or validation score: Relevant to uncertainty analysis is the use of validation scores for the input and output of the tool to determine the validity and reliability of the tool's output

2.7 Uncertainty Analysis

(1) Accuracy band

- For example, if NRW is 21% with an accuracy of +/- 66%, then the actual NRW ranges between 7% and 35%
- Guess on accuracy: In practice, no detailed information is available about data accuracy and reliability. However, water utilities can provide competent guesses about the accuracy band of certain input data (Alegre et al., 2007)

Dataorigin	Description	Accuracy bands
Measuredvolumes	System input, metered consumption, metered export	± 0.1 to 2.0%
Estimated volumes	Unmetered consumption, apparent losses	± 5 to 50%
Derived volumes	Non-revenue water, real losses	Depends on accuracy of measured and estimated input data

[Example of the Relationship Between Data Origin and Data Accuracy]

Source: GIZ (2011)

2.7 Uncertainty Analysis

- Production meter #1: The accuracy of production flow meters is critical to the calculation of system NRW
 - Different meter types render varying levels of accuracies, as shown in the following table
- Production meter #2: Over time, these meters can be influenced by several factors, such as water quality, pipe vibration, dirt entering the meter, and electronic malfunction
 - Utility managers should regularly check the accuracy of the electronic functionality of the meter (if electronic) and volumetric accuracy

Equipment/Method	Approximate Accuracy Range					
Equipment/Method Approximate Accuracy						
Electromagnetic Flow Meters	<0.15 -0.5%					
Ultrasonic Flow Meters	0.5 - 1%					
nsertion Meters	<2%					
Mechanical Meters	1.0 - 2%					
/enturi Meter	0.5 - 3%					
Neirs in open channels	10 - 50%					
/olume calculated with pump curves	10 - 50%					

[Indicative Examples of Meter Accuracy]

Source: WBI (2007)

2.7 Uncertainty Analysis

(2) Uncertainty analysis

- Uncertainty analysis. Water balance is frequently accompanied by uncertainty analysis because it is associated with uncertainties
- Method: Uncertainty of water balance can be calculated in a straightforward manner using error propagation theory

	Data origin*	Volume (m ^a)	Accuracy Ba	nd [%]	Standard deviation $\boldsymbol{\sigma}$	Variance V = (r ²
System input Q,	(M)	1,996,139	± 1.0	÷	10,184	103,721,650
Revenue water O _{rw}	(M)	1,801,146	± 0.2	<i>→</i>	1,838	3,377,891
Non-revenue water Q _{NRW}	(D)	194,993	± 10.4	÷	10,349	107,099,541
Unbilled authorised consumption Q_{UA}	(E)	30,000	± 20.0	\rightarrow	3,061	9,371,095
Water losses Q _{wL}	(D)	164,993	± 12.8	÷	10,792	116,470,637
Apparent losses O _{AL}	(E)	32,999	± 50.0	<i>→</i>	8,418	70,862,896
Real losses Q _{RL}	(D)	131,994	± 20.3	4	13,687	187,333,533

* With: (M) = metered, (D) = derived, (E) = estimated volumes.

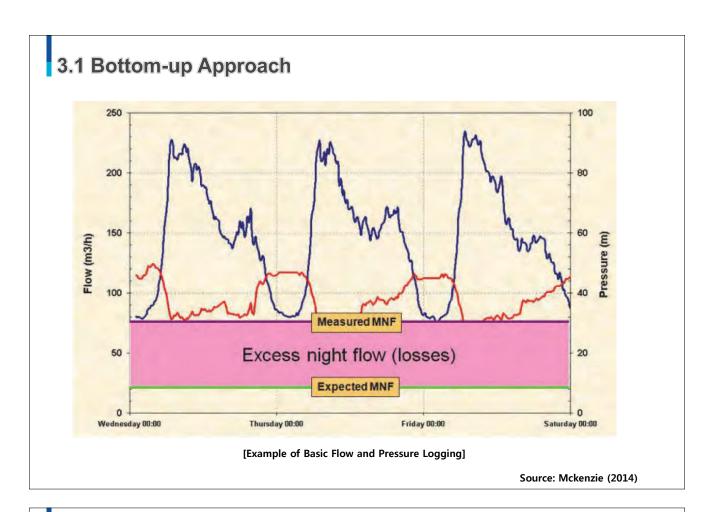
[Example for the Determination of Accuracy Bands]

Source: Liemberger (2008)



3.1 Bottom-up Approach

- Small-scale assessment #1: DMA and night flow analysis (bottom-up method) are two major tools used for the bottom-up assessment of distribution system leakage
 - To adopt the bottom-up approach, the network should be progressive and properly structured and equipped in accordance with normal operation, which can analyzed through permanently sectored district metered areas (DMAs)
- Small-scale assessment #2: The bottom-up approach, when applied in cases of sectorized systems and equipped with continuous monitoring, denotes the possibility of calculating the volume of real loss from the night flow valve
- Accompaniment: The bottom-up analysis based on night flow measurements is recommended as an complement to the annual water balance to develop an appropriate strategy for water loss reduction
- Crosschecking: The bottom-up assessment is a useful tool for crosschecking the real loss volume obtained from the top-down water balance and component analysis

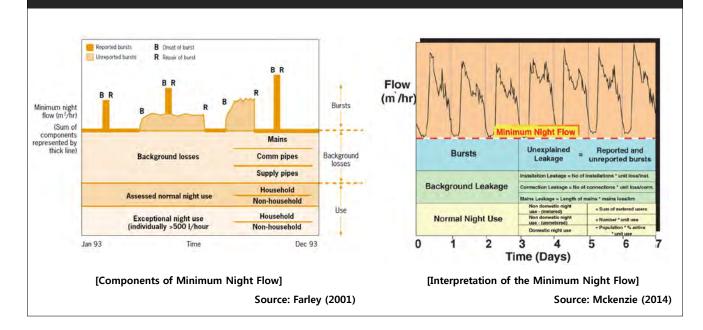


3.2 Monitoring Minimum Night Flow

Items	Details						
Time	 In most situations, the minimum night flow occurs between 02.00 and 04.00 h (Farley, 2001) MNF typically found occurs between 12 am and 4 am when the consumption in the network is at the lowest level (Mckenzie, 2014) 						
Logging		average period for each measurement, is typically 15 earch Ltd. & WRc, 1994). Correction factors can be					
frequency		minimum night flows to 1-hour flow equivalents					
frequency							
frequency							
frequency	used to change non-1-hour	minimum night flows to 1-hour flow equivalents					
frequency	used to change non-1-hour Measurement period 15 minutes 30 minutes	minimum night flows to 1-hour flow equivalents Multiplier for net night flow 1.02 1.01					
frequency	used to change non-1-hour Measurement period 15 minutes	minimum night flows to 1-hour flow equivalents Multiplier for net night flow 1.02					

3.3 Interpretation: BABE Methodology

 Components of MNF: This approach considers the MNF to consist of three main components, namely, normal legitimate night use, background loss, and burst pipes



3.3 Interpretation: BABE Methodology

- (1) Normal legitimate night use (legitimate night flow)
- Using allowances #1: Allowances for night use are made, which are derived from a sample in each category of customer because monitoring all consumers is impossible

Components	Equations
Normal domestic night use (unmetered)	 - 1.7 l/property/h or 0.6 l/head/h (UKWAR & WRc, 1994) - 6% of the population * 10 l/h (Mckenzie, 2014)
Non-domestic night use (unmetered); small non-domestic night use	- 8 l/p/h (UKWAR & WRc, 1994); simple method - 0.7–60.6 l/p/h according to SIC codes (UKWAR & WRc, 1994)
Non-domestic night use (metered); large users	

3.3 Interpretation: BABE Methodology

Using allowances #1: Managing Leakage-Report E (1994) gives a table of average values of night flow delivered to non-household customers

- It is based on one company's survey of 3,000 customers
- The customers are classified in standard industrial classification (SIC) codes and in five groups of customers with similar average uses

Category	Customer type	Night use allowance (I/hr)
A	Unmanned fire/police stations, telephone exchanges, banks, churches, chapels, gardens/ allotments, market gardens, water and sewage treatment works	0.7
В	Shops, offices, craft centres, launderettes, depots, large domestic properties, guest houses, garages/filling stations, touring caravan sites, farms, smallholdings, cattle troughs	6.3
С	Hotels, schools/colleges, restaurants, cafes, public houses, social halls, residential caravan sites, livery stables	10.4
D	Hospitals, factories, public toilets, works sites	20.7
E	Old peoples homes, mines, quarries	60.6

[Non-household Night Use Categories]

Source: UKWAR and WRc (1994)

3.3 Interpretation: BABE Methodology

(2) Background leakage

Background leakage can be related with UARL

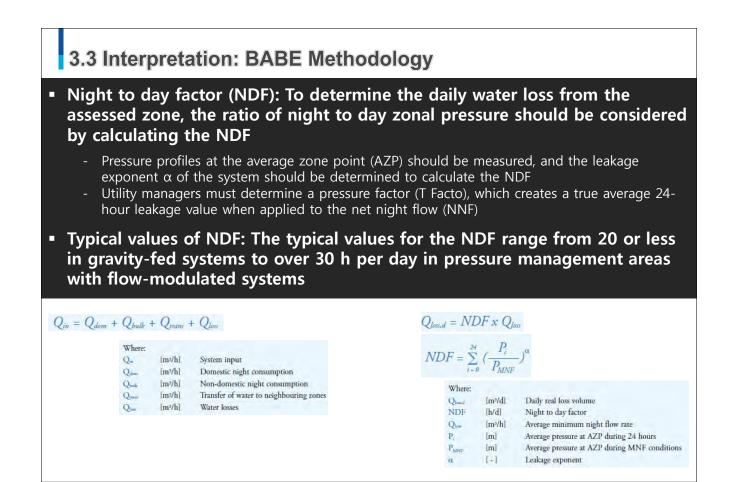
Components	Equations
Background leakage from each kilometer of mains	 0.04 l/km/h (UKWAR & WRc, 1994) Normal diameter water mains (Mckenzie, 2014): * 40 l/km/h * New pipelines: 20 l/km/h * Old pipelines: 100 l/km/h or higher
Background leakage from each connection	- None (UKWAR & WRc, 1994) - 3 l/property/h (Mckenzie, 2014)
Background leakage from each property	- ICF * PCF * 4 l/property/h (UKWAR & WRc, 1994) - 1 l/property/h (Mckenzie, 2014)

(3) Calculation	of Unexpl	lained Bur	sts				
						t is attributable e during the cal	
Description	Value	Description	Calculation	Value	_		
Construction of the second sec	Value 9 300 m	Description	Calculation	Value 18 m ³ h	_		
ength of mains	1000	Domestic night, use	3 000 @ 6%h @ 10 I	1.8 m ³ /h			
Length of mains Number of connections	9 300 m						
Description Length of mains Number of properties Estimated population	9 300 m 600	Domestic night, use Small non-domestic use Large non-domestic use	3 000 @ 6%h @ 10 I 30 @ 50 lh	1.8 m ³ /h 1.5 m ³ /h	۱.	Description	Value
Length of mains Number of connections Number of properties	9 300 m 600 672	Domestic night, use Small non-domestic use Large non-domestic use Total	3 000 @ 6%h @ 10 I 30 @ 50 lh 1 @1.2 m ³ h normal night use	1.8 m ³ h 1.5 m ³ h 1.2 m ³ h 4.5 m3h	Δ	Description	Value 2.84 m ³ /s
ength of mains Number of connections Number of properties Estimated population	9 300 m 600 672 3 000	Domestic night, use Small non-domestic use Large non-domestic use Total	3 000 @ 6%h @ 10 I 30 @ 50 lh 1 @1.2 m ² h	1.8 m ³ h 1.5 m ³ h 1.2 m ³ h 4.5 m3h		Expected background leakage	2.84 m ³ /h
ength of mains kumber of connections kumber of properties istimated population werage zone night pressure (AZNP)	9 300 m 600 672 3 000 50 m	Domestic night, use Small non-domestic use Large non-domestic use Total	3 000 @ 6%h @ 10 I 30 @ 50 lh 1 @1.2 m ³ h normal night use	1.8 m ³ h 1.5 m ³ h 1.2 m ³ h 4.5 m3h		Expected background leakage Expected normal night use	2.84 m ³ /h 4.50 m ³ /h
ength of mains lumber of connections lumber of properties istimated population vertage zone night pressure (AZNP) desured mainum night flow (MNF) lackground iosses from mains	9 300 m 600 672 3 000 50 m 14.4 m ³ /h	Domestic night, use Small non-domestic use Large non-domestic use Total	3 000 @ 6%h @ 10 I 30 @ 50 lh 1 @1.2 m ³ h normal night use	1.8 m ³ h 1.5 m ³ h 1.2 m ³ h 4.5 m3h		Expected background leakage Expected normal night use Total expected night use	2.84 m²/h 4.50 m²/h 7.34 m³/h
ength of mains lumber of connections lumber of properties stimated population verage zone night pressure (AZNP) leasured minimum night flow (INNF) leasured minimum night flow (INNF) adoground losses from mains adoground losses from connections	9 300 m 600 672 3 000 50 m 14 4 m ³ /h 40 likm/h	Domestic night use Small non-domestic use Large non-domestic use Total	3000 @ #%nh @ 101 30 @ 501h 1 @ 1.2 m ³ h normal night use [Estimate of No	1.8 m ³ h 1.5 m ³ h 1.2 m ³ h 4.5 m ³ h rmal Use]		Expected background leakage Expected normal night use Total expected night use Measured minimum night flow	2.84 m ³ /h 4.50 m ³ /h 7.34 m³/h 14.40 m ³ /h
ength of mains umber of connections umber of properties stimater population verage zone night pressure (AZNP) eassured minimum night flow (INNF) ackground issees from nains ackground issees from properties	9 300 m 800 672 3 000 50 m 14 <i>u</i> m ¹ /h 40 likm/h 3 likonnection/h	Domestic night use Simal non-domestic use Large non-domestic use Total	3 000 @ 6%sh @ 101 30 @ 50 lh 1 @ 1.2 m ² h normal night use [Estimate of No Calculation	18 m ³ /h 15 m ³ /h 12 m ³ /h 4.5 m3/h rmal Use] Value		Expected background leakage Expected normal night use Total expected night use	2.84 m²/h 4.50 m²/h 7.34 m³/h
ingth of mains umber of connections umber of properties alimated population revage zone night pressure (AZNP) easured mininum night flow (MNF) ackground cosses from nonne ackground cosses from properties of population active during night flow exercise	9 300 m 600 672 500 50 m 14 4 m ³ h 40 km ³ h 3 Wonnetionh 1 Wonnetion h	Domestic night use Small non-domestic use Large non-domestic use Total Description Mains losses	3 000 @ 6%/h @ 101 30 @ 50 l/h 1 @12.m'h normal night use [Estimate of No Calculation 9.3 km @ 40 l/km/h	1.8 m³/h 1.5 m³/h 1.2 m³/h 4.5 m3/h rmal Use] Value 0.37 m²/h		Expected background leakage Expected normal right use Total expected night use Messured minimum night flow Unaccounted-for leakage (14.40 - 7.34)	2.84 m²/h 4.50 m²/h 7.34 m²/h 14.40 m²/h 7.06 m²/h
ength of mains umber of connections umber of properties stimated population recept zone night pressure (AZNP) leasured minimum night flow (MNF) ackground iosses from mains ackground iosses from properties ackground iosses from properties ackground iosses from properties uantity of water used in tollet cistern	9 300 m 600 672 3 000 50 m 14.4 m ² h 40 kmh 3 lionneclionh 1 lionneclion h 6%	Domestic night use Small non-domestic use Large non-domestic use Total Description Mains losses Connection losses Property losses	3 000 @ 6%xh @ 101 30 @ 50 kh 1 @ 1.2 m ³ h normal night use [Estimate of No Calculation 9.3 km @ 40 km/h 600 @ 3 kconnection ih	1.8 m³/h 1.5 m³/h 1.2 m³/h 4.5 m3/h rmal Use] Value 0.37 m²/h 1,80 m²/h		Expected background leakage Expected normal night use Total expected night use Measured minimum night flow	2.84 m²/h 4.50 m²/h 7.34 m²/h 14.40 m²/h 7.06 m²/h
ength of mains Jumber of connections Jumber of properties stimated population verage zone right pressure (AZNP) Measured mnimum night flow (MNF)	9 300 m 600 672 3 000 50 m 14 4 m ³ /h 40 likm/h 3 liconnection/h 6% 10 l	Domestic night use Small non-domestic use Large non-domestic use Total Description Mains losses Connection losses Property losses	3 000 @ 6%xh @ 101 30 @ 50 kh 1 @1.2m ² h normal night use [Estimate of No Calculation 9.3 km @ 40 kkm/h 600 @ 3 loonechon in 672 @ 11 lyopertyin teleakage at 50 m pressure	1.8 m ³ /h 1.5 m ³ /h 1.2 m ³ /h 1.2 m ³ /h 4.5 m³/h Value 0.37 m ³ /h 0.67 m ³ /h		Expected background leakage Expected normal right use Total expected night use Messured minimum night flow Unaccounted-for leakage (14.40 - 7.34)	2.84 m²/h 4.50 m²/h 7.34 m²/h 14.40 m²/h 7.06 m²/h

3.3 Interpretation: BABE Methodology

(4) Daily Leakage

- Snapshot of leakage: The leakage calculated for the MNF period will not be a true representation of leakage across a 24-hour period
 - Leakage is proportional to the pressure in the system
 - Water flow in DMA is different; therefore, head loss is not constant during day





4.1 Performance Indicators and Benchmarking

- Requirement of PIs: Performance indicators numerically evaluate different aspects of the distribution system and should be consistent, repeatable, and presented in meaningful standardized units
- Importance: The aspect of NRW measurement and benchmarking has been one of those important interventions, which many water utilities has been pursuing and developing over the years
- NRW and infrastructure leakage index (ILI): In developed countries, NRW is used as a financial indicator instead of an operational indicator, whereas the ILI is used as a technical performance indicator for real loss (Kelay et al., 2006)
 - Although a percentage of NRW can provide a high-level financial perspective of the water utility, it is not a good operational PI and is unsuitable for effective management of water loss (AWWA, 2003; Fanner et al., 2007; Liemberger et al., 2007)

4.1 Performance Indicators and Benchmarking

(1) Functions & advantages

- Function #1: A PI system can be considered a key assessment tool for the achievement of targets by applying a coherent set of indicators
 - PIs and benchmarking are identified as appropriate tools for the evaluation of the performance of the water supply systems
 - PIs extract necessary information from large amounts of data and are useful tools for decisionmaking processes
 - Different PIs should be calculated and analyzed to determine whether these losses are comparatively high or low
 - However, PIs are not solely tools for national or international benchmarking between water supply utilities and should never be a goal in themselves
- Function #2: Calculating performance indicators enables a comparison between the management of utilities in different countries and of varying dimensions. They are relevant as a decision-making tool and aim for continuous improvement and knowledge cultivation regarding the supply system conditions and performance

4.1 Performance Indicators and Benchmarking

(2) Limitations

- Percentage: A general consensus worldwide is that percentages should not be used because they can be extremely unreliable and frequently provide wrong illustrations
 - Despite the general international agreement on the non-use of percentages, they remain the indicator of choice in most parts of the world
 - If the use of percentages is acceptable worldwide, then they should be accompanied by a form of warning and, if possible, at least one of more reliable indicators should be included

(3) Requirement

 Requirement of PIs: Performance indicators should be clear and easy to understand and calculate and has a rational basis

4.2 Classification

(1) Level

PIs can be classified into three categories (i.e., basic, intermediate, and detailed)

			Service connection density			
Level	Function		>20/km of mains	<20/km of mains	Comments	
1—Basic	Financial	NRW	Volume of NRW as % of SIV	Volume of NRW as % of SIV	Simple, not recommended	
1—Basic	Operational	AL	m ³ /serv. conn./year	m ³ /km of mains/year	For target setting, not comparing systems	
1—basic	Operational	RL	L/serv. conn./day	L/km of mains/day	For target setting not comparing systems	
1—Basic	Operational	RL	L/serv. conn./day w.s.p.	L/km of mains/day w.s.p.	Allows for intermittent supply situations	
2—Interm.	Operational	RL	L/serv. conn./day/m pressure	L/km of mains/day/m pressure	Useful for comparing systems	
3—Detailed	Financial	NRW	Value of NRW as % of annual cost	Value of NRW as % of annual cost	Allows different unit costs	
3—Detailed	Operational	RL	ILI	ILI	Powerful for comparing systems	

4.2 Classification

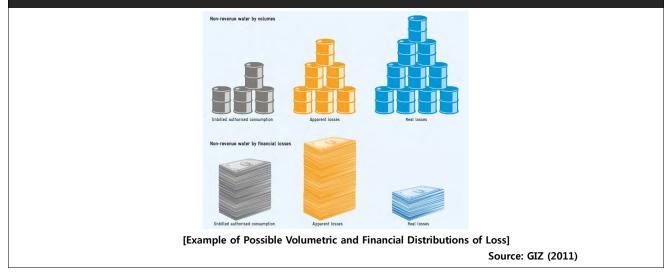
(2) PIs for financial, operation, and water resources

- One of the most commonly used approaches in defining performance indicators is understanding their subdivision into district groups, such as financial, operational, and water resources
- The following tables were prepared by the IWA Water Task Force and outlined details related to the most commonly used PIs (EPAL, 2017)

Component	Туре	Performance Indicator base	Performance Indicator detail
Non-Revenue Water	Financial	NRW volume as % of water volume in system	NRW system value as % of system cost
Water Losses	Operational	m ³ /service connection/year	•
Apparent Losses	Operational		m ³ /service connection/year
Real Losses	Water Resources	Real losses volume as % of water volume in system	
Real Losses (in each case, this indicator is calculated "per day" when the system is pressurized to allow for the effect of intermittent supply	Operational systems	Litres/service connection/day for systems with 20 or more service connection per km of main or use m ³ /km/day for sys- tems with less than 20 service connection per km.	Infrastructure Leakage Index (ILI): defined as the ratio between current annual real losses and unavoidable annual real losses = CARL / UARL.
[H	Key Performanc	e Indicators Related to W	ater Loss]

4.2 Classification

- Comparing components from the financial perspective: Water utilities should first focus on the components of NRW where investments will generate the highest rate of return
 - Comparing the components of NRW not only by volume but also by financial impact is important
 - A small volume of commercial loss may have a higher financial value. Therefore, if the objective is to increase financial resources, then commercial losses should be prioritized



4.2 Classification

(3) PIs for total, real, or apparent loss

• PIs can consider the aspects of total, real, and apparent losses

Object	Details
Total loss	- NRW, NRW%
Real loss	 Liter per service connection per day (l/c/d) Liter per service connection per day per meter of pressure (l/c/d/m pressure) Liter per kilometer of pipeline per day (l/km/d) Infrastructure leakage index (ILI)
Apparent loss	- Apparent loss index (ALI) - Percentage of authorized consumption

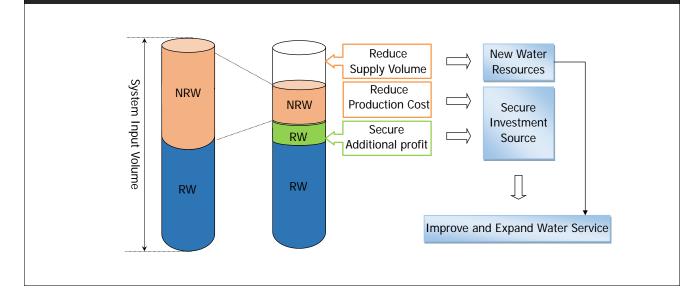
4.3 PI Selection Process

- No general rule: Selecting appropriate performance indicators to quantify and monitor leakage from a water network is frequently contentious
- Contextualization: Given the diversity of situations that occur in water supply networks and the intrinsic characteristics of their functions, contextualizing several key concepts inherent to the calculation of the abovementioned indicators is important to ensure a uniform perception

4.3 PI Selection Process PI selection 1st Step: Define precise and clear objectives and determine the most suitable strategies 2nd Step: Select appropriate PIs and necessary input data collected **Objectives** Objectives Establishing continuous water supply. Reducing NRW to target level. Which results are to be reached in the future? Strategies Strategies 1. New metering program 2. Pressure management How can those results be reached? Critical success factors Critical success factors 1.a Replacement of defective meters 1.b Improve meter-reading 2. Design and implementation of new PMA Depending on constraints and context Monitor water tosses per service connection and day. Monitor changes in average pressure and service time. Have the objectives been reached? What happened with the critical success fectors? [Use of PIs in Decision-making Processes: General Approach and Example] Source: Alegre et al. (2007)

4.4 NRW

- Definition: Non-revenue water is defined as the water produced and lost before reaching consumers
- Effect: High levels of NRW are detrimental to the financial viability of water utilities and the quality of water itself



4.4 NRW

- Operational and financial Pis: NRW is a good indicator of water utility performance. NRW is a measure of a utility's efficiency in terms of operational and financial performance
 - High levels of NRW typically indicate a poorly managed water utility

 Widely accepted PIs: Managers, policymakers, regulatory agencies, and financing institutions use NRW PIs to rank the utility's performance against industry standards and those of other water utilities

4.4 NRW

- Deviance from the exact procedure #1: Published NRW data are typically problematic, suspicious, or inaccurate or provide partial information
 - Many utilities invent "creative" definitions of NRW, use wrong or misleading performance indicators, and fail to quote important information, such as average pressure and supply time
- Component division of NRW: In many utilities, the exact breakdown of NRW components and sub-components is simply unknown; thus, decision making on the best course of action for NRW reduction is difficult
- Deviance from the exact procedure #2: Creative definitions for NRW are used to reduce the reported numbers (e.g., minimum leakage or unbilled consumption is deducted to wrongly reduce the volume of NRW)

- Wrong numbers are frequently also reported to meet regulatory requirements

4.5 Benchmarking

Benchmark (BM): A performance indicator (or collection of several indicators) can be used to establish a benchmark A BM enables a PWS to evaluate its performance over a period of time by repeating performance-indicating tests and comparing them with previous results PIs and BMs enable comparisons between public water providers Interval: Benchmarking can be conducted at any increment of time: daily,

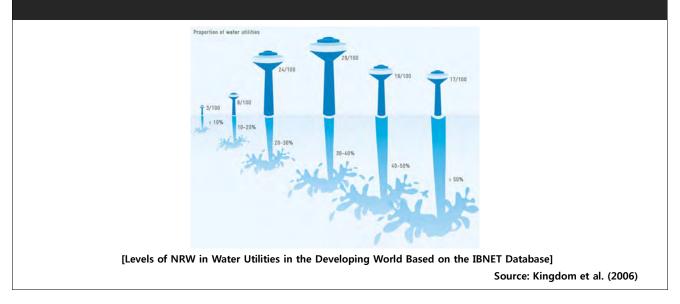
- Evaluating its performance
 - Identifying areas for improvement
- Comparing itself to other water systems

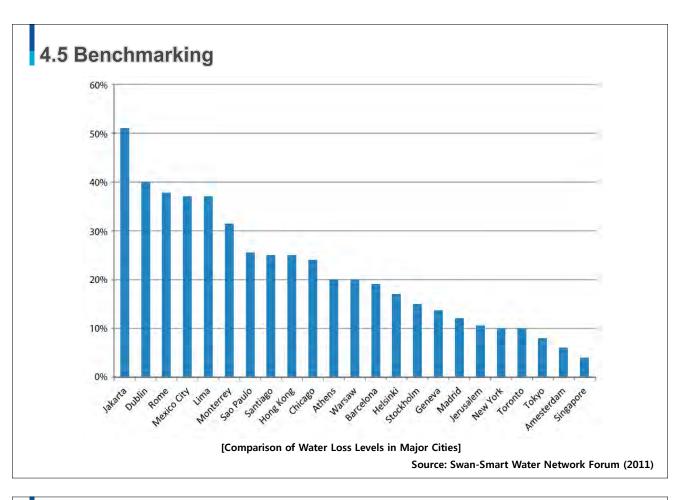
monthly, yearly, or every few years

- Evaluating financial options
- Gauging itself in a competitive manner
- Providing data for reports to the public, regulators, and, ultimately, water users
- Difficulty in ELL: Deciding the acceptable level of water loss is a difficult task because it is dependent on the specific conditions of each utility (operational, i.e., network length, connection density, and service pressure, and commercial)

4.5 Benchmarking

- Example of BM: The World Bank estimates that the actual figure for over NRW level in developing countries ranges from 40% to 50% of water produced
 - Based on a study on 40 water utilities in South-East Asia and IBNET database on water utility performance for over 900 utilities in developing countries

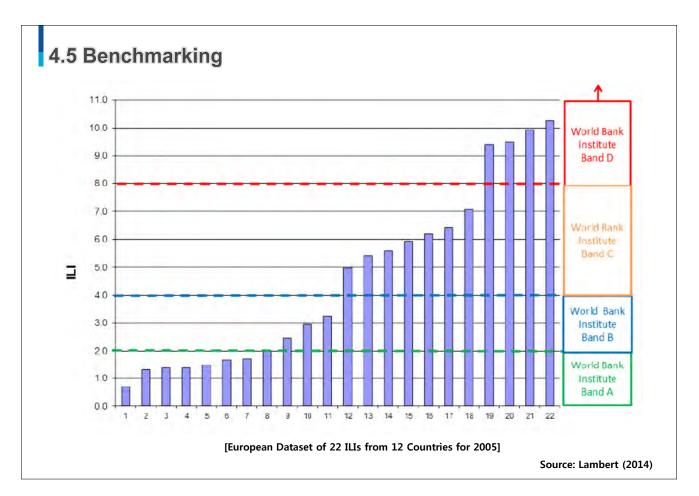




Techr Perform	ance	ILI			in Litres/Co ssurised); at		-
Categ	ory	1-21	10 m	20 m	30 m	40 m	50 m
	Α	1 - 2		< 50	< 75	< 100	< 125
Developed Countries	в	2 - 4		50 - 100	75 - 150	100 - 200	125 - 250
Jevelopec Countries	С	4 - 8		100 - 200	150 - 300	200 - 400	250 - 500
	D	> 8		> 200	> 300	> 400	> 500
-	Α	1 - 4	< 50	< 100	< 150	< 200	< 250
Developing Countries	в	4 - 8	50 - 100	100 - 200	150 - 300	200 - 400	250 - 500

[Physical Loss Target Matrix (from WBI NRW Training Module 6: Performance Indicators)] Source: Liemberger (2007)

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5. Tools and Software

Out of the 21 tools reported in the literature, 12 are freely available (Al-Washali et. al., 2020)

Many (commercial) tools are designed to assist water utilities assess losses and plan reduction interventions

	Tool (version)	Reference	Developer	Environment	Description	Approach	Scale
1	AquaLite (v4.5)	(Mckenzie, 2007)	WRC	Windows-based	A tool to establish WB and PIs	Top-down	Global
2	AWWA Water Audit (v5)	(Water Loss Control Committee, 2014)	AWWA	Excel-based	A tool to establish WB and PIs. Uses validity score (qualitative), not uncertainties	Top-down	Global
3	BenchLeak	(Mckenzie, Lambert, Kock, & Mtshweni, 2002)	WRC	Excel-based	A tool to establish WB and PIs	Top-down	Global
4	BenchLoss (v2a)	(GWR-Ltd, 2008)	GWR	Excel-based	A tool to establish WB and Pls	Top-down	Globai
5	CalacuLEAKator (v4,3)	(Koldžo & Vuc'ijak, 2013)	Djevad Koldzo	Excel-based	A tool to establish WR and FIs, based on MNF analyses	MNF	DMA, global
0	CheckCalcs (v6b)	(Lambert, 2015a)	ILMSS Ltd.	Excel-based	A tool to establish WB and Pls. Provides insights on leakage relationships, N ₁ , N ₂ , N ₁	Top-down	Global
7.	Component Analysis	(Sturm et al., 2014)	WRF	Excel-based	WB and PIs are inputs. Analyses potential of leakage reduction interventions	Top-do., BABE, PM, ALC	Global
8	EconoLeuk (vla)	(Mckenzie & Lambert, 2002)	WRC	Excel-based	A tool to establish the ELL with cost-benefit analysis of ALC	ELL.	Giobal
9	PresMac (v4.4)	(Mckenzie & Langenboven, 2001)	WRC	Windows-based	Operational tool for pressure management in a DMA, using PRVs	BABE, FAVAD, PRVs	DMA
10	SanFlow (v4.6)	(Mckenzie, 1999)	WRC	Windows-based	A tool to model MNF in a DMA and breakdown leakage into components	MNF, BABE	DMA
11.	WB-EasyCalc (v5.16)	(Liemberger and Partners, 2018)	Roland Leimberger	Excel-based	A tool to establish WB and PIs. Analyses impacts of changes in pressure, SIV, and supply time	Top-down	Global
12	WB-PI Cale-UTH (v2.2)	(Tsitsifi & Kanakoudis, 2010)	Tsitsifi and Kanakoudis	Excel-based	A tool to establish WB and Pls. Considers the overbilling practices in the balance	Top-down	Global

[Free Software Tools for NRW Assessment]

Source : Al-Washali et al. (2020)



Apparent loss management

Water Loss Management



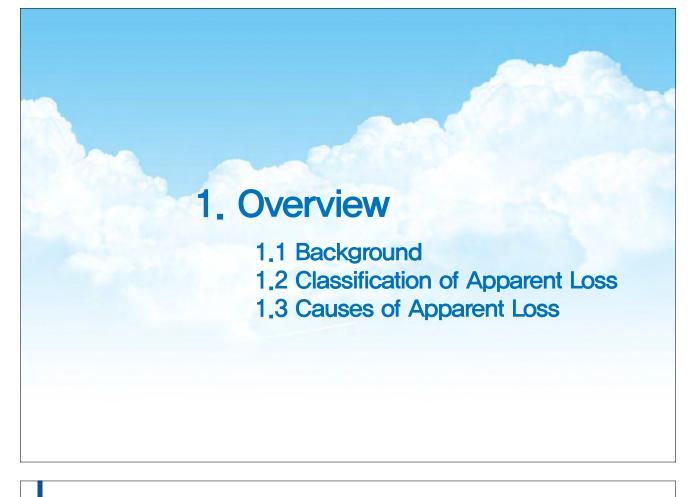


Aims and Objectives

- The course aims to:
 - (1) Provide trainees with activities for controlling apparent loss;
 - (2) Enable trainees to establish an apparent loss management plan based on results of water audit
- The objectives are that trainees will understand:
 - (1) Activities to reduce data handling errors;
 - (2) Activities to reduce unauthorized consumption;
 - (3) Activities to manage meter inaccuracy

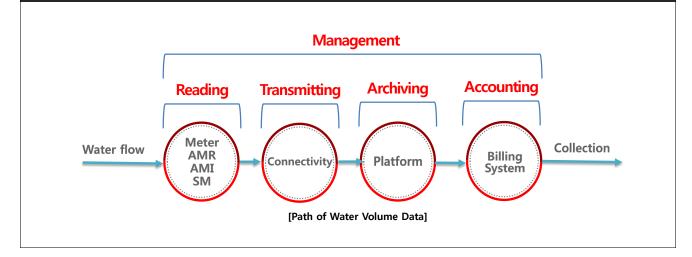
Contents

- 1. Overview
- 2. Apparent Loss Management Programs
- 3. Quantifying Apparent Loss
- 4. Management of Data Handling Errors
- 5. Management of Unauthorized Consumption
- 6. Management of Meter Inaccuracy



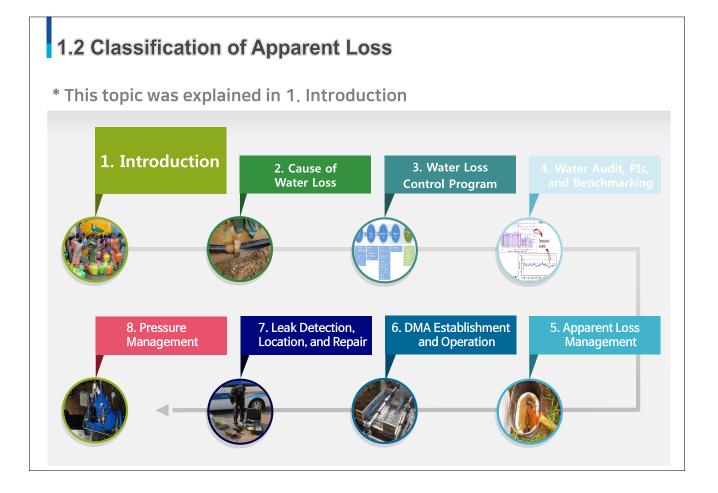
1.1 Backgrounds

- Apparent Loss: Apparent loss occurs as a result of inefficiencies in the management, reading, transmitting, archiving, and accounting operations to track water volume in the water utility
- Influencing factors: Apparent loss can be influenced by social, cultural, political, and financial factors and may require organizational and institutional changes to reduce apparent loss



1.1 Backgrounds

- Rigorous procedures: Procedures for installing, registering, maintaining, reading, and billing for all new meters should be rigorous
- Low-hanging fruits: Reducing apparent loss can be achieved in many cases at relatively low costs and is therefore typically a good starting point with a quick pay-off to the water utility
- Amount of apparent and real losses: Apparent loss is nearly always less in volume than real loss. However, it does not mean that apparent loss reduction is less important









Apparent loss management program: The following figure identifies a sequence of steps for developing and implementing the apparent loss control program after the compilation of the initial top-down water audit

- After quantifying apparent loss, the management of data handling error, unauthorized consumption, and meter inaccuracies is implemented one by one



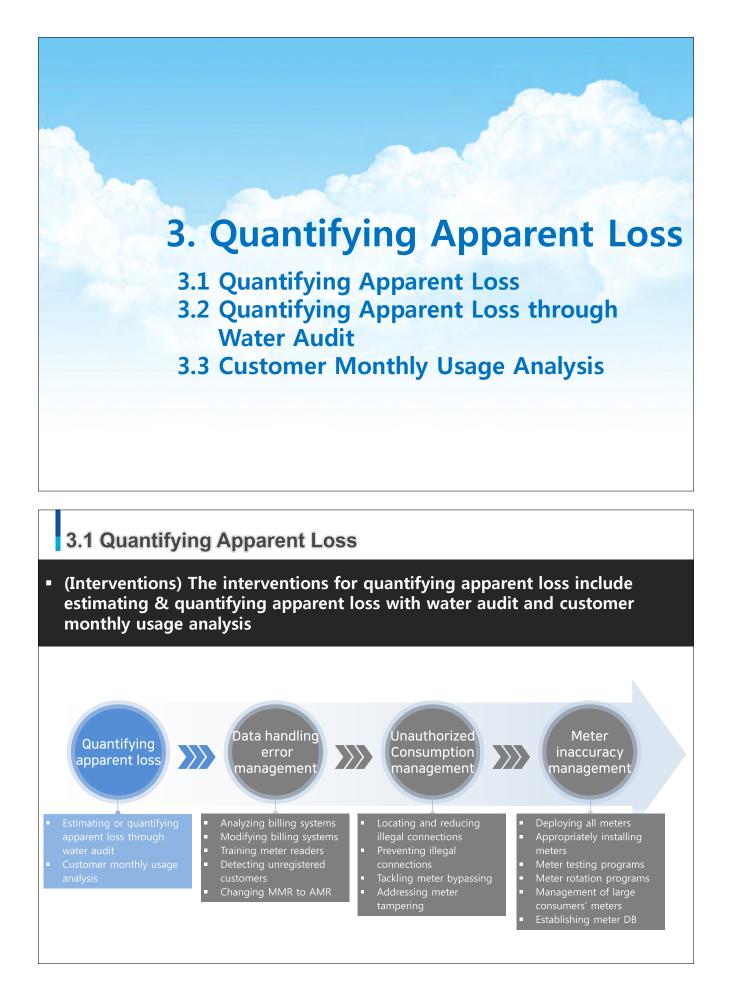
2. Apparent Loss Management Program

Intervention order (first option): Reducing data handling error is the first step in apparent loss management

- A recommendation is that the billing system analysis should be performed as the initial step because integrity in this process can influence data used in the other steps

Intervention order (last option): Replacing meters is the last resort in apparent loss management

- Many water utilities tend to assume that the apparent loss in their system is solely caused by customer meter inaccuracy and that replacement of the entire customer meter population is the best intervention for reducing apparent loss
- Implementing a comprehensive customer meter changeout is a very expensive and inefficient proposition if the bulk of the apparent loss cases was in fact caused by billing system data errors or unauthorized consumption
- However, many water utilities continue this scheme and become perplexed when, after spending millions of dollars on new meters, the apparent loss standing remains the same



3.2 Quantifying Apparent Loss with Water Audit

(1) Water audit

- Understanding the characteristics of apparent loss: The auditor should first assemble the water audit and identify the nature, quantity, and cost impact of the three components of apparent loss in each water supply system
- Establishing a strategy: Establishing a strategy for apparent loss control based on the results of the water audit is the best way to proceed
- Understanding the dynamic characteristics: Water utilities should understand that apparent losses are not only multidimensional but also dynamic in nature

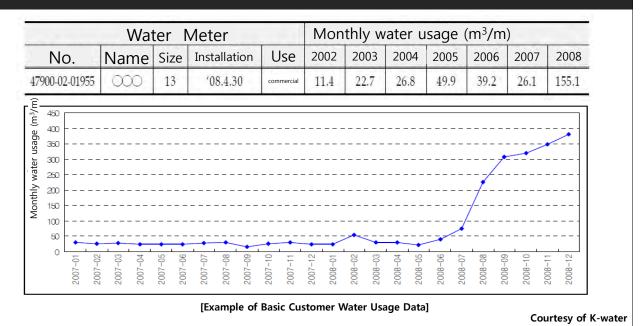
3.2 Quantifying Apparent Loss Through Water Audit

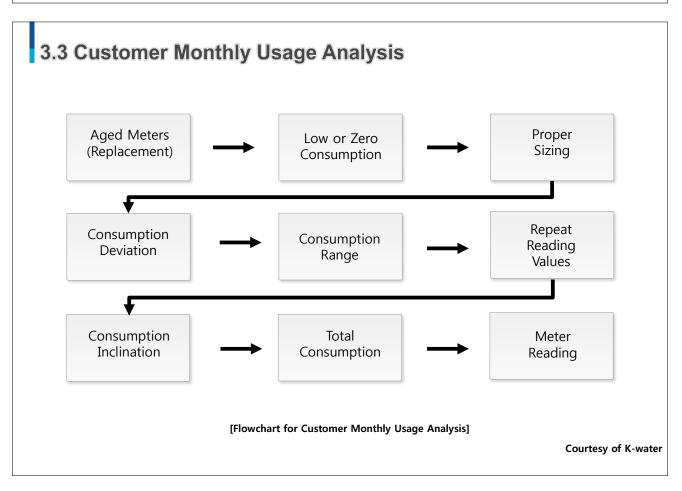
(2) Data validation and bottom-up water audit

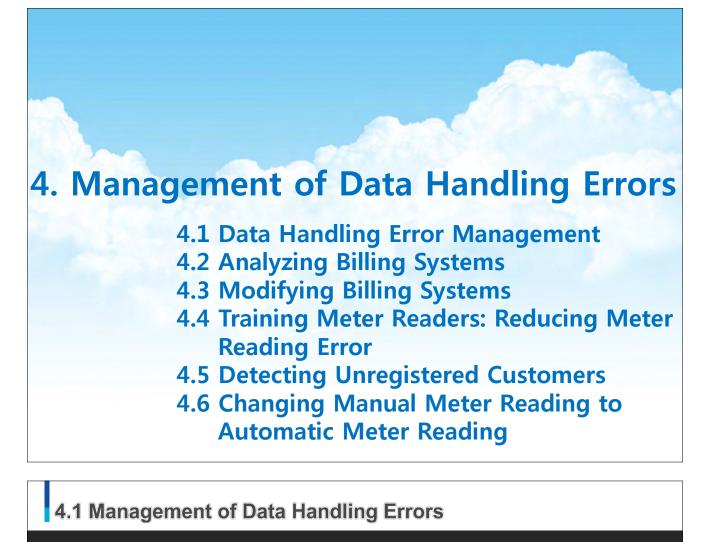
- Data validation: Before setting a definitive strategy, auditors should perform a detailed investigation of the source data or functions to validate the preliminary data and obtain an accurate picture of apparent loss
- Bottom-up process: The bottom-up process involves a detailed investigation and auditing work, which is similar to the detailed financial audits performed by accountants
- Quantifying exact values: If the three components of apparent loss are calculated with estimated values in the top-down audit, then the quantification of these components is performed in the bottom-up audit

3.3 Customer Monthly Usage Analysis

Identifying issues for each customer: Prior to the on-site survey, the standardized customer monthly usage analysis should be used to identify the problems of each customer







 Interventions: The interventions for data handling error management include analyzing and modifying billing systems, training meter readers, detecting unregistered customers, and changing manual meter reading to automatic meter reading



4.1 Management of Data Handling Errors

 Processes where errors occur: Data handling errors occur during reading, transmitting, archiving, and reporting customer consumption total values

- Considerable data handling errors can occur during the customer meter reading process

- Fast payback: Although data handling errors can be subtle and require considerable investigative time to detect, corrections are typically quick and inexpensive and requires only minor procedural or programming changes
- Increasing data integrity: Addressing data handling flaws early in the water loss control program creates a foundation for data integrity that is essential as the loss control program matures

4.2 Analyzing Billing Systems

- Analyzing billing systems: Billing systems can be analyzed by outlining the flow paths of billing data and documenting information-handling policies, procedures, practices, and programming
- (Outline data handling process) Constructing a series of flowcharts that outline the information data handling process is a systematic approach that can reveal gaps in policy, procedures, practices, or programming



CHECKING POLICIES

 Policies regarding customer metering, billing, water rates, and customer service connection piping responsibilities



CHECKING PROCEDURES

- Existence of written procedures
 Procedures to ensure consistent metering, meter reading, and billing functions
- meter reading, and billing functions



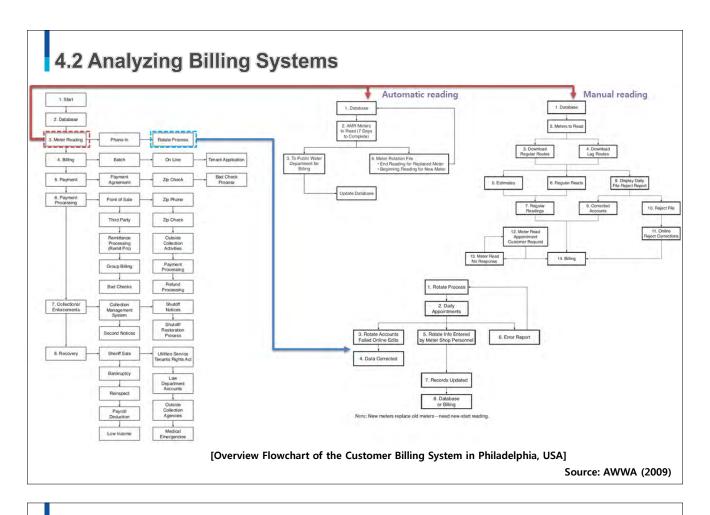
CHECKING PRACTICES

- Actual practices that reflect the mandates of the procedures
- Whether meter readers, billing clerks, or similar employees are properly monitored and supervised to detect and minimize human error



UNDERTAKING A FLOWCHARTING EXERCISE

 Undertaking a flowcharting exercise of the billing process to identify any impacts to the integrity of customer consumption as well as any apparent loss components from the data handling process



4.3 Modifying Billing Systems

 Modifying the system: Water utilities should modify the sources of apparent loss as a result of data handling errors to establish a highly detailed picture of the billing process



CORRECTING DB

All properties are on the billing system, are billed, and payments are up to date
Utilities ensure that all customers/properties are on the correct tariff



SEPARATING FINANCIAL AND ENGINEERING SYSTEM

 Although a negative consumption value can be acceptable for billing (financial) purposes, it is relatively harmful to the integrity of the data for operational (engineering) purposes

4.4 Training Meter Readers: Reducing Meter Reading Errors

- Contribution: The content of training meter readers can contribute to immediate reporting, implementation of new procedures, and verification of the customer database
- Benefits: Training meter readers promotes diligent, good customer meter maintenance, and decreased meter reading errors

IMMEDIATE REPORT OF PROBLEMS - Meter readers should immediately report

- Meter readers should immediately report any observed problems
- The maintenance team should take immediate action to remedy the problem
- If remedial action is slow, then meter readers may become demoralized and less inclined to report problems

VERIFYING CUSTOMER

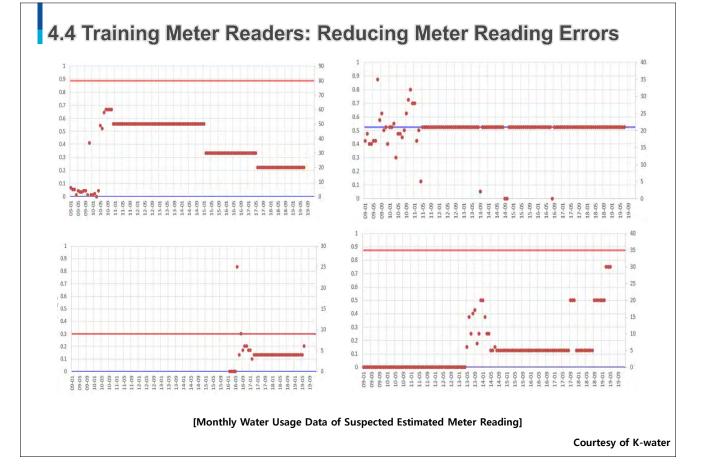
DATABASE

- During routine meter reading, meter readers can be instructed to confirm the supply status of consumers



IMPLEMENTING NEW PROCEDURES

 Meter reading errors are addressed by implementing new systems for meter reading and billing



4.5 Detecting Unregistered Customers

 Registering service connections: Detecting unregistered connections and registering in DB are required tasks because intentionally and accidentally omitted service connections can cause apparent loss

DETECTION DURING A REGULAR METER READING CYCLE

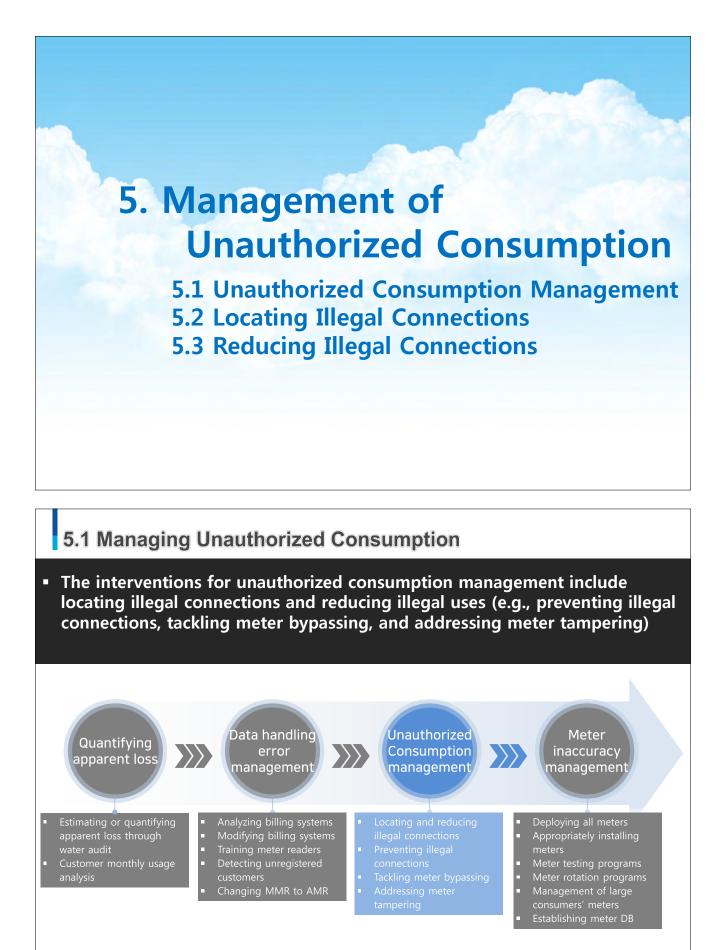
- Unregistered customers can be detected during a regular meter reading cycle when diligent meter readers find meters that are not listed in their reading book
- However, this process may not identify all errors in the billing system



CONDUCTING A COMPLETE CUSTOMER SURVEY

- Conducting a complete customer survey within each DMA, where utility representatives visit every property in the DMA, is the best method for comprehensively identifying errors in the billing system
- The survey should include the following information: property address, name of owner, and meter make and number
- The representative should also conduct a meter test to ensure that accurate flow is recorded

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5.1 Management of Unauthorized Consumption

- This is more of a technical problem: Although other problems in NRW management are technical, water theft is a political and social issue
- Support from the local government: Reducing illegal use is beyond the scope of the utility because it requires a high level of political support
 - Reducing this part of apparent loss is neither technically difficult nor costly but requires making difficult and unpleasant managerial decisions that may be politically unpopular
- Frequently related to the poor: Regularization of illegal connections, which often occur in poor areas, will impact the poor in many cases
- Reflecting utility level: The value that the community and water utility place on water supply and the management of the effectiveness of the water utility are often reflected by the amount of unauthorized consumption that occurs within a locale

5.2 Locating Illegal Connections

 Detection: Water utilities should employ mechanisms for detecting trends of unauthorized consumption



BILLING DATA REVIEW

 Billing data should be reviewed for suspicious trends that may reflect unauthorized consumption



CHECKING FIRE HYDRANTS

- Utilities should review opportunities for the unauthorized use of fire hydrants and ensure the existence of a rational policy regarding fire hydrants

03

HOUSEHOLD INSPECTIONS

 Household inspections can be conducted on select zero consumption accounts



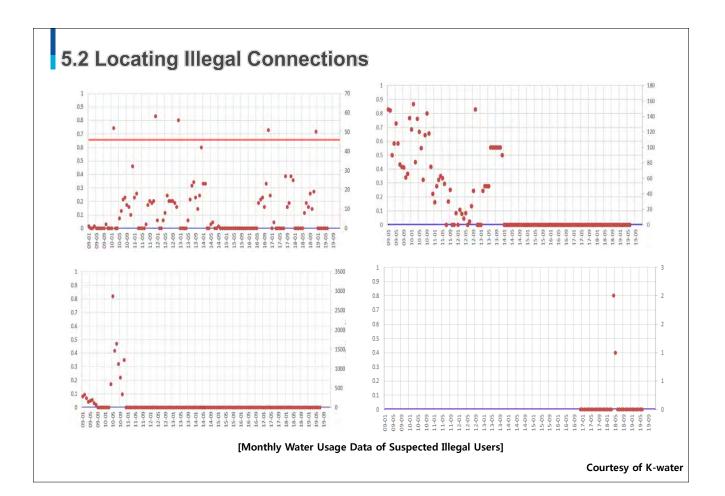
INSPECTION OF SHUTOFF CUSTOMERS

- If utility policies enable customer service to be terminated because of payment delinquency, then follow-up random inspections should be conducted to ensure that customers have not reactivated their service illegally

05

BOUNDARY VALVE

Boundary valves for neighboring water systems should be inspected periodically to ensure that they are in the proper positions



5.3 Reducing Illegal Connections

• Effective enforcement: To control unauthorized consumption on a long-term basis, the water utility should employ effective policies and enforcement capabilities



CHANGING REGULATIONS

A strong legal framework will ultimately allow the water utility to operate with enforcement powers to maintain unauthorized consumption to an economic

LOCKABLE VALVE

- Lockable valves limit illegal connections



USING BETTER QUALITY METER

Most reputable meter manufacturers produce meters that are tamper-resistant with non-metallic parts, strong clear plastic windows, and impenetrable casing



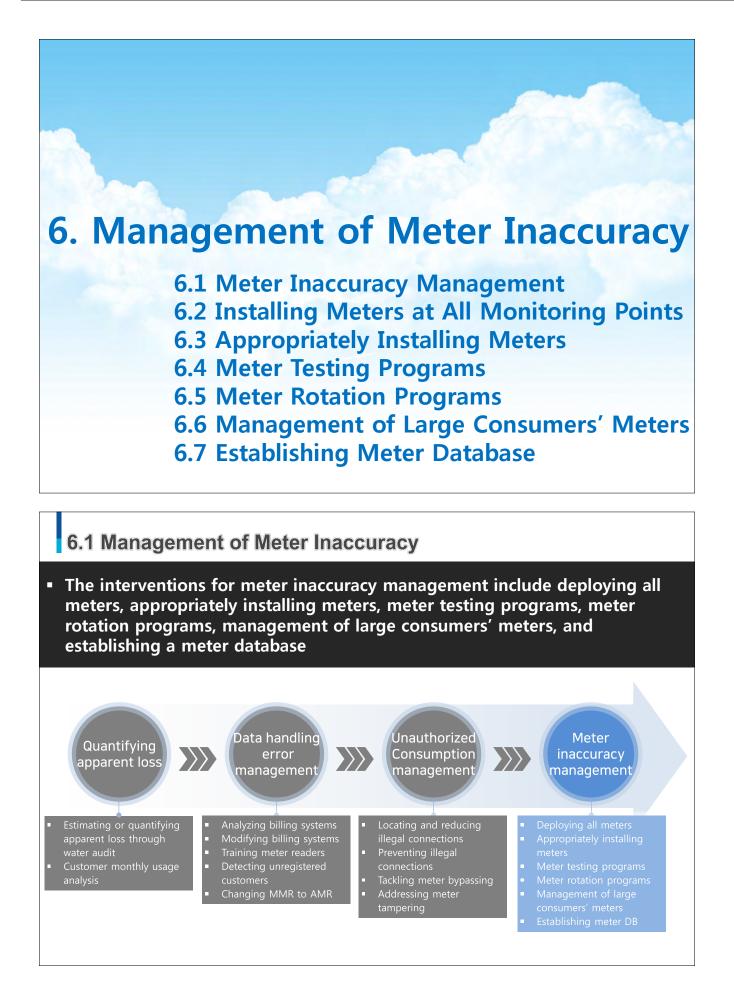


REVIEW POLICIES AND PROCEDURES

- Review service connection policies and procedures to ensure that they do not encourage illegal connections
- Simplify connection application procedures

LEAKAGE STEP TEST

The utility should undertake customer surveys and leakage step tests to determine where the missing flow occurs



6.1 Management of Meter Inaccuracy

- Critical part in revenue: A well-managed customer meter scheme is critical to the revenue stream of the water utility
- Understanding of meters: Managing a large population of customer meters requires knowledge of meter and meter reading equipment as well as customer relations
- Understanding of policy: This understanding is required to verify the current policy of universal metering and metering programs as follows:
 - Guidance on which meter to install and when to replace it
 - A hierarchy of metering application exists in the majority of water supply systems

6.1 Management of Meter Inaccuracy

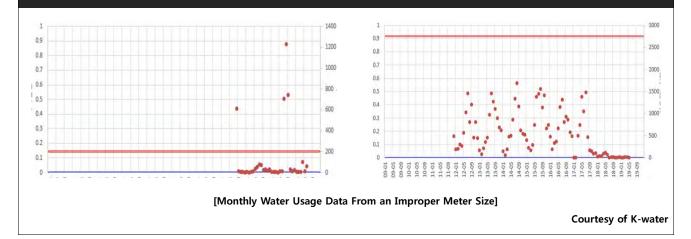
- Maintenance cost: The upkeep of the meter population requires expenditures for installation, maintenance, calibration, testing, and replacement
- Lack of experience: An incoming water utility manager with a lack of experience in ongoing meter testing, rotation, or right-sizing is expected
- Poor documentation: The size, type, make, and performance of the meter population is poorly documented and understood

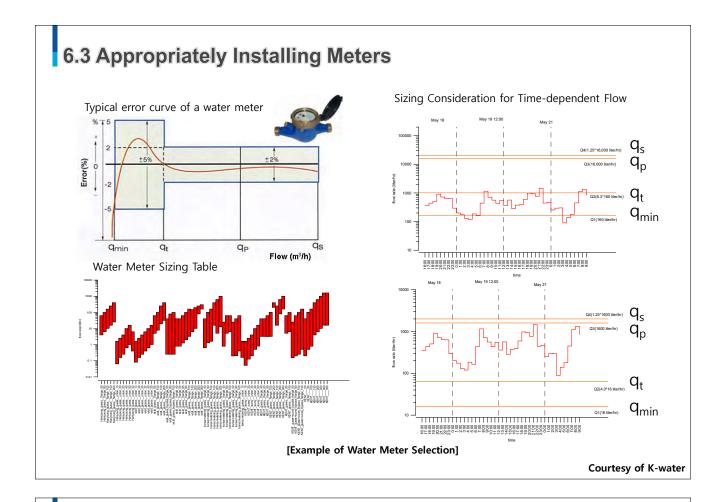
6.2 Deploying All Meters

- Fixed tariff scheme without meter: Many small utilities charge a flat monthly rate for water and may meter only several large-use customers, if at all
 - Flat rates may be based on the type of use, such as residential, commercial, industrial, or agricultural or based on occupancy
- Deploy all meters: General meter management exists in the context of water utilities with a fully metered customer population
- Representative sample monitoring: Systems that do not meter their customers can obtain an approximation of customer consumption. Metering and data-logging representative samples of customer accounts can be used, and the results are statistically evaluated to infer the general trends of consumption

6.3 Appropriately Installing Meters

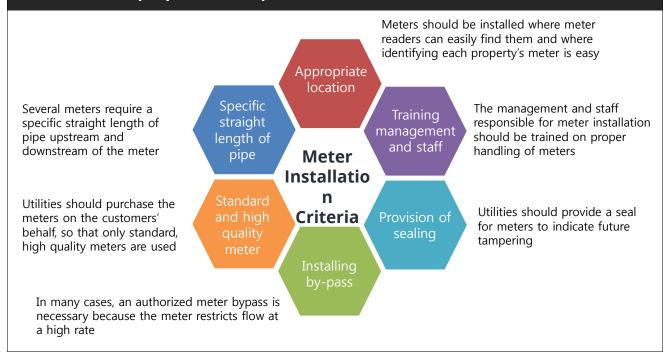
- Appropriate metering: Selecting the appropriate type and size of the meter is important as well as its proper installation
- Appropriate type of meter: Selecting the appropriate meter can help ensure the accuracy of customer consumption data
- Size: Meter sizing is very important because the accuracy of the meter is dependent on the design type and design flow





6.3 Appropriately Installing Meters

 Installation: Following the manufacture's installation instructions for a meter is crucial for proper meter operation

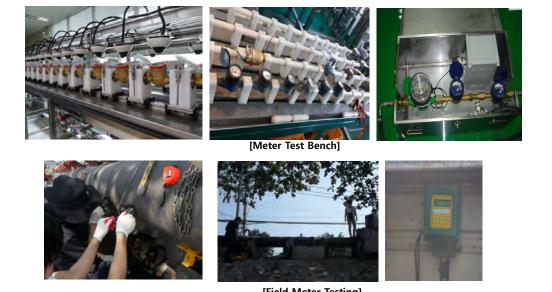


6.4 Calibrating and Testing Meters

- Meter deterioration: Mechanical meters will slow down with age and become less accurate. As such, they must be replaced or re-calibrated at regular intervals, which is every few years
- Effects: Under-registration results in lower billing and loss of potential revenue and erroneously indicates an increased level of water loss from the system at the same time, which can alter the results of water audit results
- Meter calibration process: Meters should be calibrated according to the manufacturers' instructions
- (DB) Testing results can be used as a basis for quantifying apparent loss in water audit or meter repair and replacement

6.4 Calibrating and Testing Meters

 Methods can be classified into a calibrated meter test bench or a field meter testing: comparing volume flow over time against accurate a master meter



[Field Meter Testing]

Courtesy of K-water



6.6 Large Consumers' Meter Management

- Poor maintenance: In many municipalities, metering and billing of large consumers are poor in performance with numerous meters buried or broken or old and many un-metered connections
- Importance: Utilities should focus initially on large consumers, such as industrial or commercial users, because they consume large volumes of water and pay high tariff rates
 - Revenue generated from a small number of large industrial consumers can exceed the remaining revenue generated from residential customers
- Only cost effective for large meters: Many water utilities found that calibration or repair of small residential meters is not cost-effective because the majority of meter models retain accuracy over a long time. In addition, the cost of a new meter is less than the cost of calibration or repair
 - Calibration and repair remain common for most models of large commercial and industrial meters

6.7 Establishing a Meter Database

- Meter DB: The basic demographics and accuracy levels of the meter population should be established
- Meter demographics: If the meter population characteristics are unknown, then the auditor can conduct research using purchase and installation records, billing records, customer complaint histories, and meter accuracy test results to compile information on the sizes, types, manufacturers, ages, and cumulative consumption levels of customer meters
- Testing for data: To determine the physical accuracy of the meter population, many water utilities operate their test facility and equipment and perform ongoing accuracy testing of meters that were rotated out of service



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	United Nations Educational, Scientific and Cultural Organization	i-WSSM		

DMA

Water Loss Management

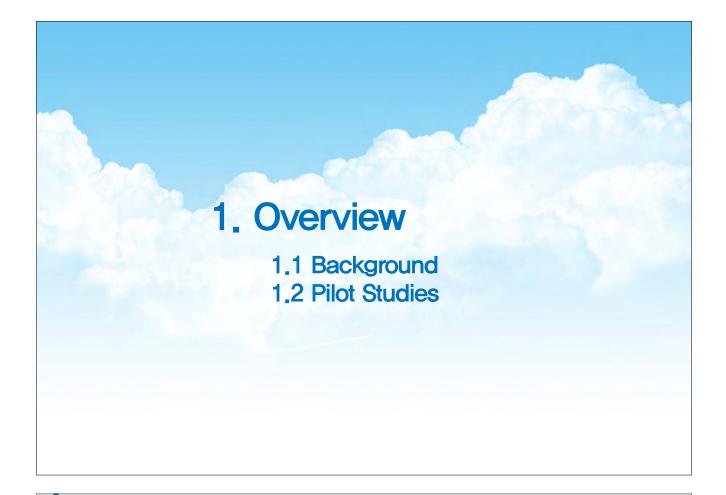




Ain	ns and Objectives
Th	e course aims to:
(1)	Provide trainees with information on establishing, operating, and sustaining DMAs;
(2)	Enable trainees to sectorize the water network based on DMA design guidelines and improve current DMA operation and maintenance
Th	e objectives are that trainees will understand:
(2)	DMA design criteria and guidelines; Monitored data analysis and intervention in DMA; DMA boundary management

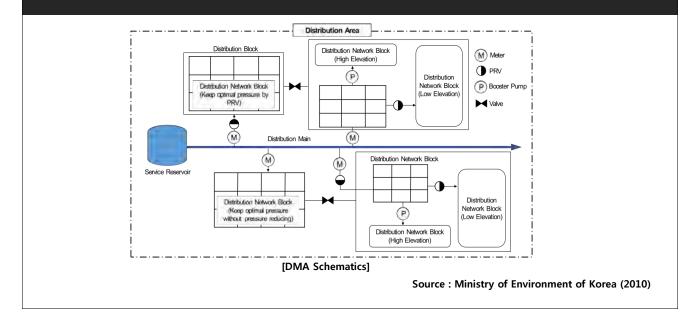
Contents

- 1. Overview
- 2. Design Guideline
- 3. DMA Establishment and Management Processes
- 4. DMA Establishment
- 5. DMA Management



1.1 Backgrounds

 A DMA is defined as a discrete sector of the distribution network, which is formed naturally or imposed and can effectively evaluate the continuous inflow of supply through flow meters installed at the input and output metering points



1.1 Backgrounds

 Benefits: Partitioning the network into DMAs yields enormous benefits, such as pressure control, network management, leakage monitoring, leakage detection, maintenance of water quality, and asset management

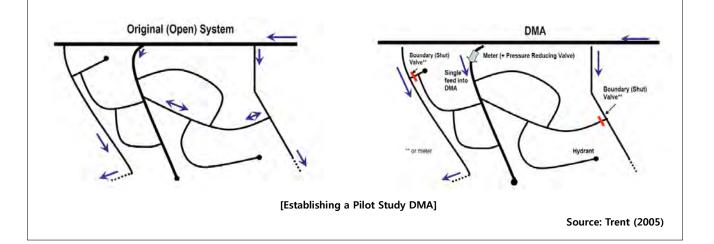
Planning	Water loss quantification	Leak detection	
 With sectorization, water utilities can easily plan operation and maintenance 	 Enables the measurement of background leakage to be distinguished from unreported leaks 	 Well-managed DMAs serve as early warning systems of newly emerging leakage and can alter the operator 	
Prioritizing vulnerable area		Pressure management	
 DMAs enable water utilities to quantify the current level of leakage in a discrete area and to consequently prioritize leak detection activities 		• The network is operated at the optimum level of pressure by managing the pressure in each district or group of districts	
Emergency response	Safeguard water supply	Continuous water supply	
 Water utilities can respond emergency by isolating the problematic DMA only 	 Establishing DMAs enable water utilities to prevent the deterioration of water quality in the distribution network 	 Although bursts occur in one DMA, water utilities can supply water to other DMAs continuously 	
	[Benefits of DMA]		

1.1 Backgrounds

- Starting point of real water loss management: Unfortunately, although sectorizing is recognized as very important, recognizing that sectorizing is only a means to an end and not the end itself is equally important
 - Only establishing DMAs do not reduce water loss. However, water utilities can easily manage water loss with established DMAs
- Increasing the knowledge of the network: The implementation of a plan for network sectoring, which aims to monitor leakage, will result in major improvements in the management and knowledge of the network
- Measures for intermittent supply: Creating DMAs may also be the first step in counteracting intermittent supply
 - DMAs facilitate the location and repair of major leaks and thus may enable the reduction of periods of supply interruption

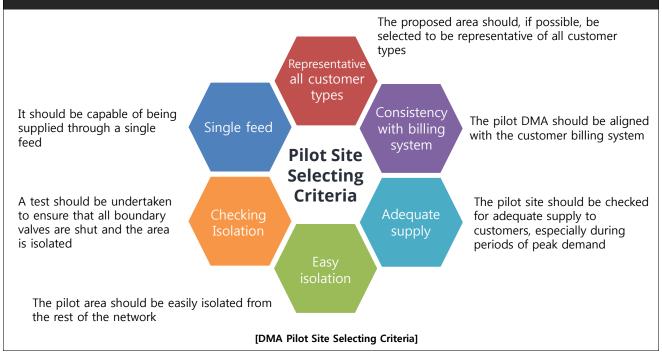
1.2 Pilot Studies

- Pilot studies on natural DMAs: For ease and experience gain, initiating the network sectorization process by implementing natural DMAs, namely, the areas of the network that were already naturally closed, is advisable
 - A suggestion is that one or two pilot DMAs be set up to implement the principles and procedures of the water loss control program



1.2 Pilot Studies

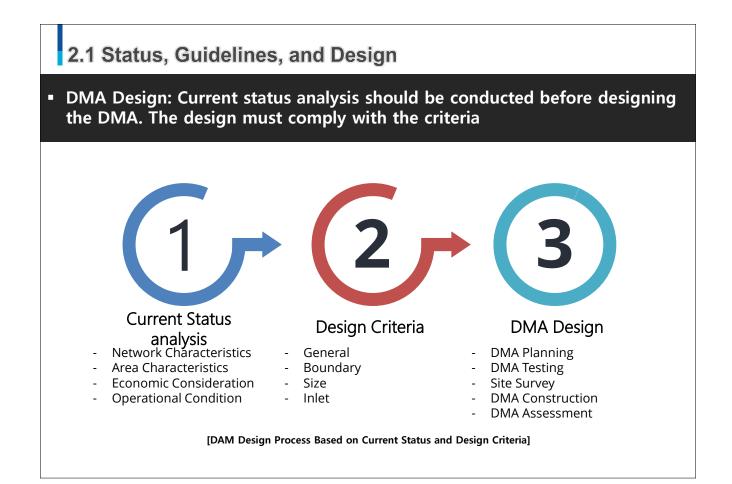
 Selecting the pilot site: Selecting the first pilot area is important and will be based on a combination of several factors



1.2 Pilot Studies

- Accumulating experience: Pilot studies are the best means for building knowledge and understanding of these areas
- Accountability: A staffing structure should be allocated to each DMA to ensure staff accountability. DMA leaders are also appointed to guarantee accountability for the management and performance of the DMA
- Performance evaluation: The pilot area will be used to measure, qualify, identify, and rectify apparent and real losses
- Scale-up pilot: The successful planning and implementation of a pilot area that produces measurable results will provide motivation to eventually extend the trials to the entire area

2. Design Guidelines 2.1 Status, Guidelines, and Design 2.2 Current Status and Information 2.3 Design Criteria



2.2 Current Status and Information

- In-depth knowledge: Designing DMAs requires in-depth knowledge of the water supply system
- Required information: A number of factors should be considered for the correct design of a DMA, such as network, area, and geographical characteristics and network operation conditions



NETWORK CHARACTERISTICS

- Existence of sensitive or critical areas for correct supply
- Existence of large customers and constraints
 Complete and up-to-date network information

GEOGRAPHICAL CHARACTERI STICS

- Geographical density of customers (population density in the supply network)
 Geographic or demographic factors, such as
 - urban, industrial, or rural areas



AREA CHARACTERISTICS

- Size and physical characteristics of the area
- Topographical information



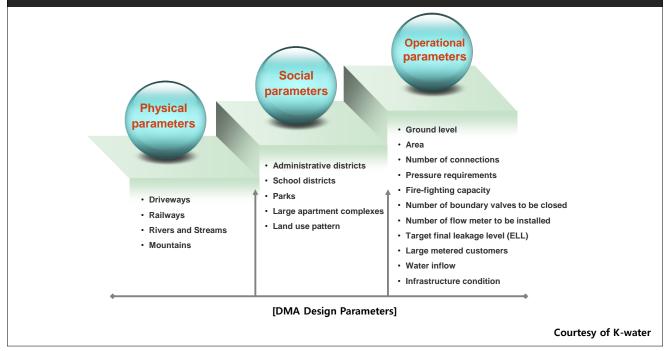
OPERATION CONDITIONS

- Water consumption pattern
- Operational data about flows and pressure
- Calibrated hydraulic model
 Techniques available or applicable for leakage control
- Types of pipe materials and age
- Service levels and water quality

[Items to be Surveyed for Current Status Analysis]

2.2 Current Status and Information

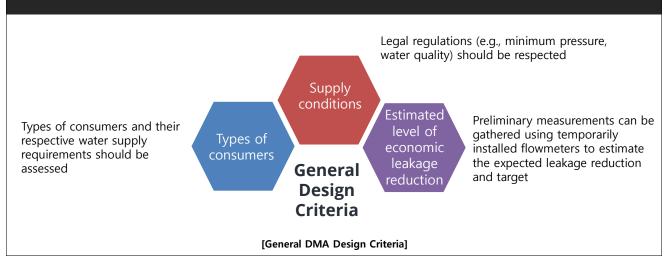


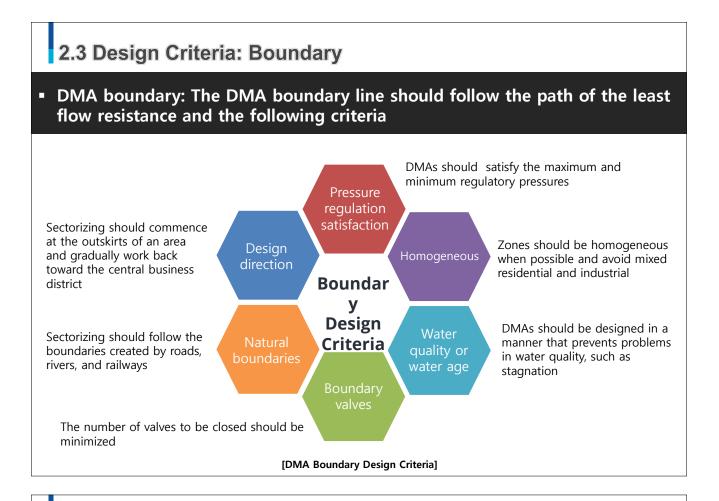


2.3 Design Criteria: General

 No general rule: The design of a series of DMAs is extremely subjective. Moreover, a case of two utility engineers working on the same network and formulating the same design is unlikely

- When establishing zones in an area, a few general rules of thumb should be followed and considered
- Each company will have its own criteria for setting the economic levels of leakage and leakage targets for each DMA





2.3 Design Criteria: DMA Size

No ideal size exists for a DMA

- The size, whether 500 or 5,000 service connections, is always a tradeoff
- The decision has to be made on a case-by-case basis

 Influencing factors: The size of an individual MDA will vary depending on local factors and system characteristics, such as

- required economic level of leakage;
- geographic/demographic factors;
- previous leakage control techniques;
- individual water company preference;
- hydraulic conditions

Significant influencing factors: An important influencing factor is the condition of the infrastructure

- If the mains and service connections are fragile, then bursts will become more frequent and the optimal DMA will be relatively small
- Conversely, in areas with brand new infrastructure, DMAs can be large and remain manageable

2.3 Design Criteria: Inlet Design

- Inlet design: The process is composed of survey information, meter site selection, meter (flow meter and PRV) selection and installation, and pipework arrangement
- Meter site selection: Once the DMA has been selected, the location of the single supply should be examined
 - Ideally, the location should not be in a carriageway
 - The water mains should be at a standard depth and be easily accessible for future meter readings and maintenance
 - The area should not be flooded in general
 - If vandalism is rife, consideration should be given to the use of secure chamber lids

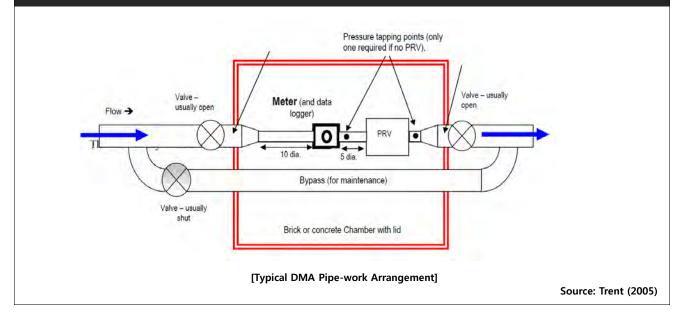
• Meter selection: Installing the correct size and type of meter is important

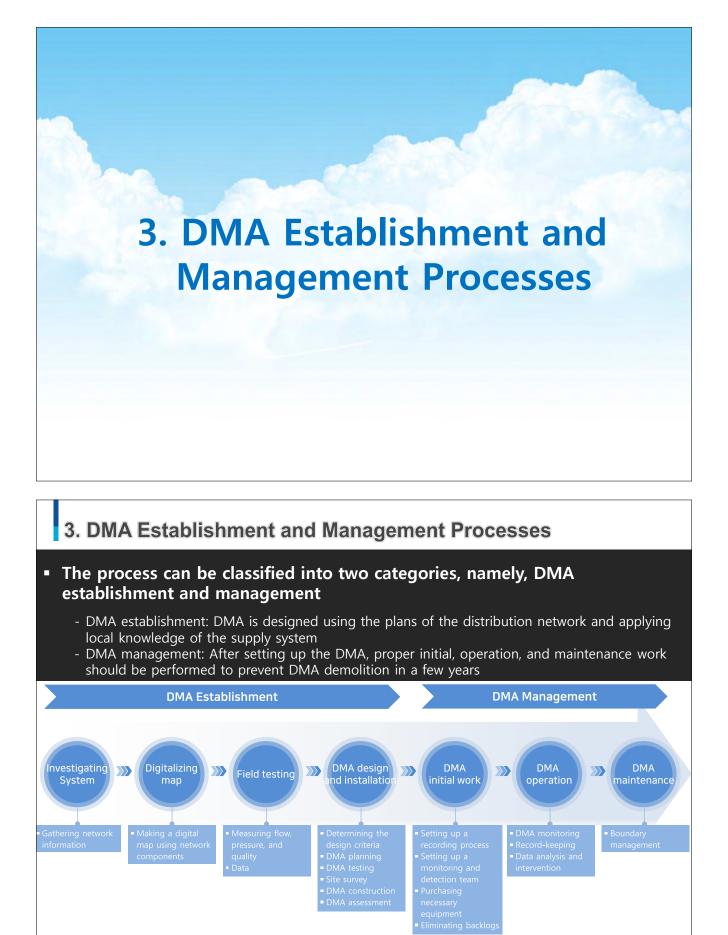
- When determining the dimensions of the flow meter, the following should be considered: existing pipe diameter, expected flow range, admissible head loss at peak flow, and reverse flow requirements

2.6 Design Criteria: Inlet Design

 Pipe-work arrangement: The meter (and PRV if required) should be fitted in a water-tight chamber with safe access to meter reading and maintenance and with substantial lids

- Positioning the PRV within the meter chamber is not always possible





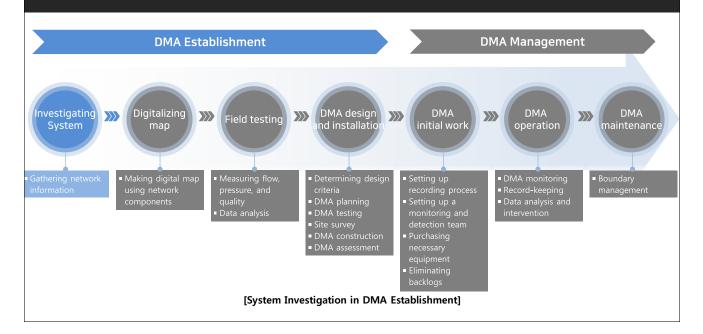
[Process of DMA Establishment and Management]

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4. DMA Establishment 4.1 System Investigation 4.2 Map Digitalization and Updating 4.3 Field Testing 4.4 DMA Design and Installation

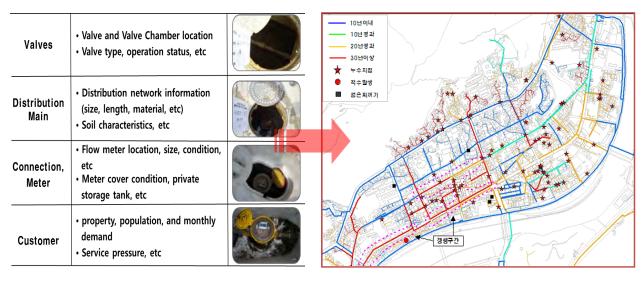
4.1 System Investigation

 Data collection: System investigation is the first step in DMA establishment. In this step, all forms of documents related to the water distribution network and its operation and maintenance should be collected



4.1 System Investigation

Items: Information related to devices (e.g., valves), distribution mains, service connections, and customers are surveyed

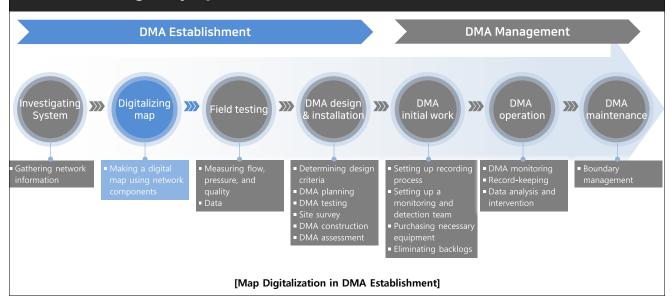


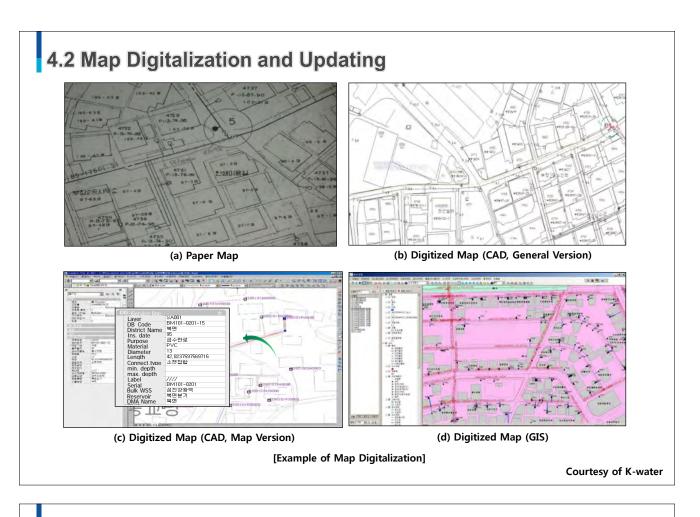
[Items for Water Network System Investigation]

Courtesy of K-water

4.2 Map Digitalization and Updating

- Map digitalization: Based on the collected data, a digitalized map should be made in the form of CAD or GIS
- Updating: Data on network components and operation and maintenance should be regularly updated

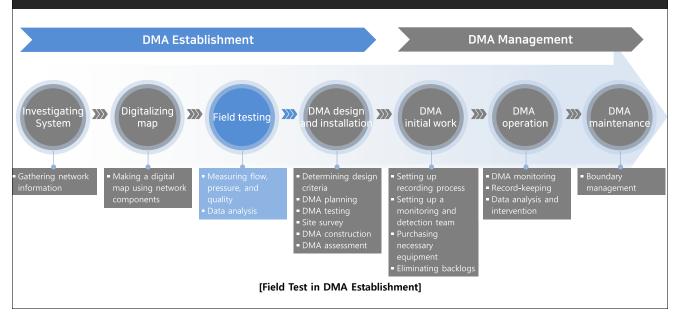


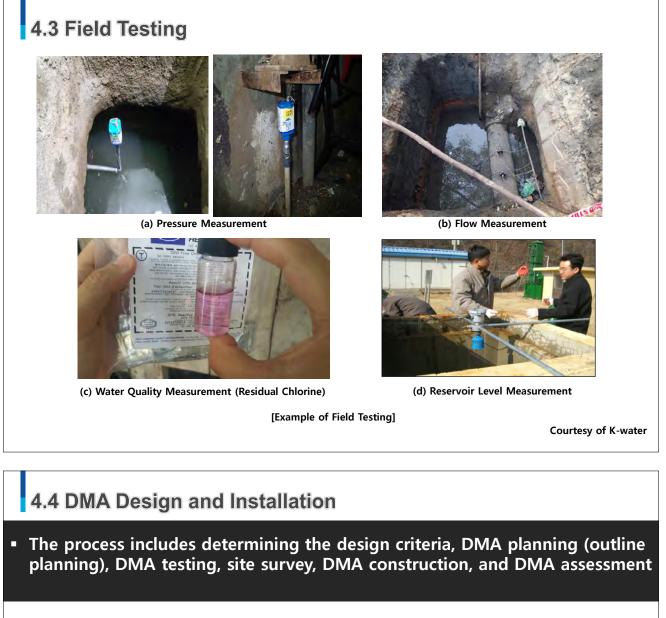


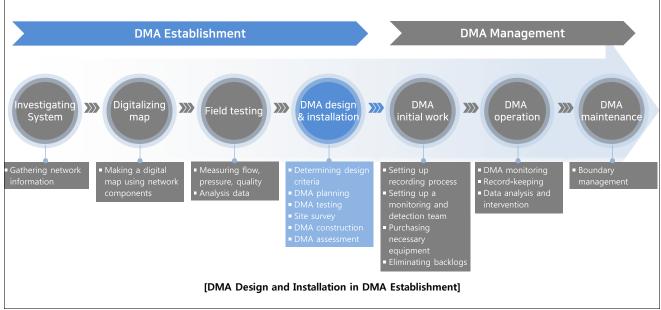
4.3 Field Testing

 Flow rate, pressure, and residual chlorine concentration are measured to analyze and diagnose hydraulic and water quality performance within a network

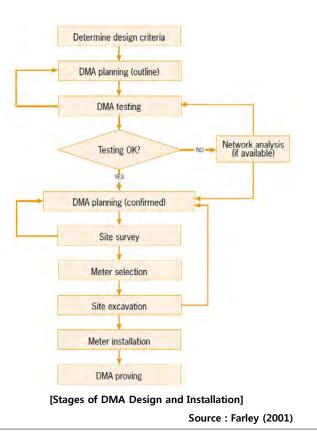
- Field testing provides an understanding of system behavior







- Process: The figure is a flow diagram that demonstrates the stages of design and installation
- Interrelated tasks: The planning and design of a DMA can be described as performing a series of interrelated tasks
- Iterative process: The overall shape is iterative, which corresponds to the development resulting from the integration of information obtained from the surveys and field investigations throughout the process



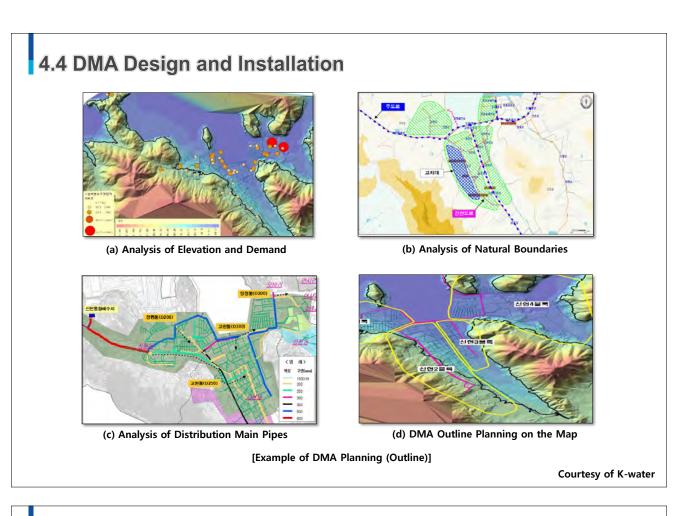
4.4 DMA Design and Installation

(1) Determining the Design Criteria

• The details were explained in Chapter 2.2

(2) DMA Planning (Outline Planning)

- Outline planning: The planning stage is the process of dividing the distribution system into suitable-sized DMAs. Outline planning is the first step, which uses small-scale distribution maps of the mains to draw provisional boundaries
- Provisional drawing: The provisional drawing of the DMA border should be prepared using GIS with the aim of characterizing the DMA and establishing procedures for continuous implementation. At the same time, the supply quality and pressure should be secured during periods of high consumption



(3) DMA Testing

- Objectives: The objectives of DMA testing are to verify whether the DMA can supply water without low pressure problems and whether the DMA can be isolated
- Checklist: Before conducting DMA testing, the valve condition should be checked

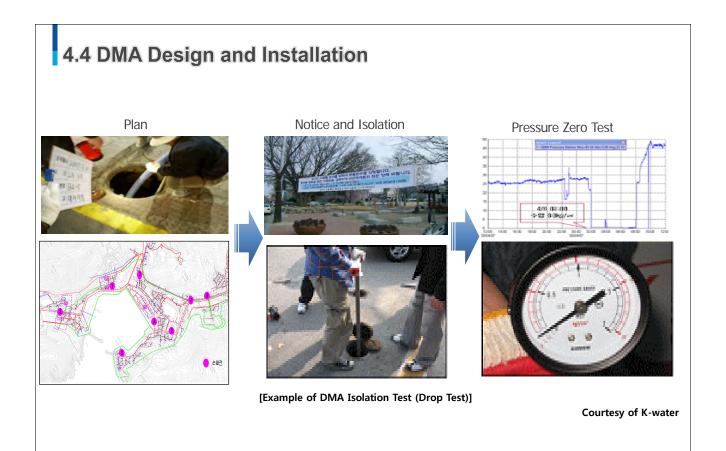
Check list	Details
Valve tightness	 Before any further work, a trial closure of the DMA boundary valves should be conducted The boundary valves for the proposed DMA should be checked for tight-closure by listening on the valve key when the valve is fully shut Once the limits of a new DMA have been determined, a site survey should be executed; the existing boundary valves should be closed and tested for tightness Testing the valves is essential because one leaky valve will distort the leak assessment not only in the actual DMA but also in the neighboring DMAs
Installing new valves	 In the absence of valves or in cases where valves do not shut tight, new valves should be fitted Defective valves should be replaced, and a new valve should be installed at boundaries without valves
Labeling	 Undertake the sealing and labelling of DMA boundary valves, such they can be easily identified on the ground by the operational and maintenance staff

- Low pressure (verifying service pressure): This process ensures that DMA pressures are maintained up to the standard of service
- Low pressure (application time): The implementation process should be applied during a period of high consumption, thus verifying that the pressure values at the critical point satisfy the regulation and guaranteeing appropriate supply to customers
- Low pressure (handling negative impact during the process): If a significantly negative impact is observed on network pressure, then a rapid opening of the DMA boundary valves should be undertaken
- Low pressure (redesign): If pressures are not maintained, then the outline planning stage should be repeated, and extra meters should be installed to replace the closed valves

4.4 DMA Design and Installation

- Isolation (drop test): After closing all boundary and inlet valves, the pressures in the DMA should be measured using recorders/loggers to ensure that the DMA is isolated
- Isolation (water quality problem): The isolation process can cause problems in water quality because it can change flow direction and velocity
- Isolation (not isolated): If the pressure does not drop or if it rises again after closing the hydrant, then a partially closed boundary valve or unknown connections to an adjacent zone are highly probable

- The area of the potential inlet can be localized by assessing the pressure head (hydraulic grade line) throughout the DMA



(4) Detailed Site Survey

 A site survey is necessary to verify the potential meter location that was decided in the planning exercise



LINE OF THE MAINS

After an accurate location of the line of the main, "walking" the main and noting and verifying the positions of valves, bends, and connections are advised



AUDITING OF EXISTING FACILITIES

This step is required to ensure that flowmeters, PRV, and boundary valves are operating normally



BYPASS INSTALLATION

Most companies install a bypass at the DMA meter site, which can be a bypass around the meter or a meter on a bypass



OTHER UTILITIES

Determining the position of other utilities is necessary, such that they can be avoided when excavating the meter site



TELEMETRY

While conducting the site survey, noting the proximity of the facilities required for the telemetered DMA outstation is beneficial

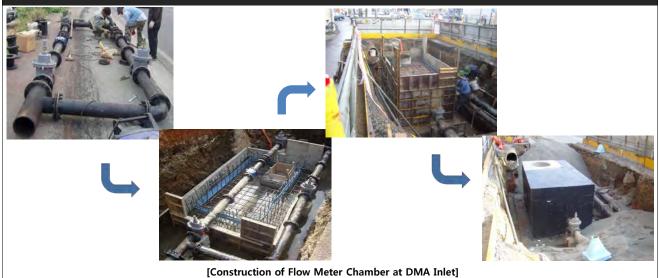


SPACE FOR METER LOGGER

Accessibility problems can be overcome by connecting the meter to a logger, which can be located in a small chamber

(5) DMA Construction

 The DMA inlet, SCADA system, and water network integrated operation system should be installed

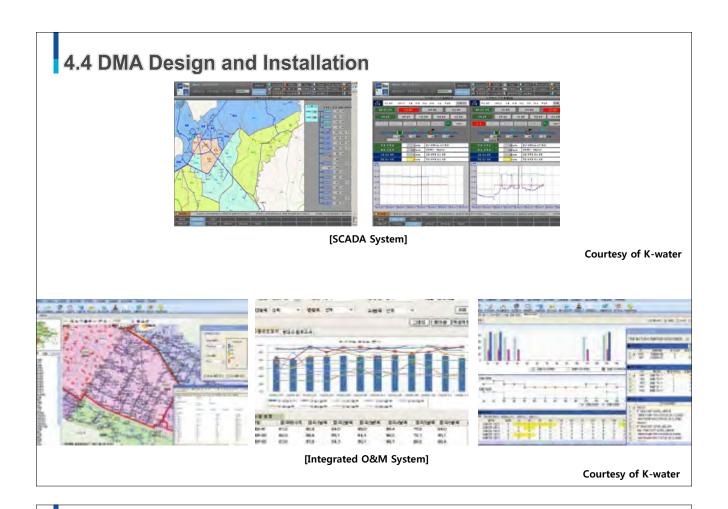


Courtesy of K-water



[Equipment for Monitoring and Control at DMA Inlet]

Courtesy of K-water



(6) DMA Assessment (DAM Proving)

- DMA assessment: In this process, low pressure and isolation are verified again, and installed equipment and flow capacity are tested
- Low pressure: If an unacceptable pressure reduction occurs in operating the DMA, then revising the DMA design may be necessary to provide sufficient pressure within the DMA
- Equipment test: After installing and testing, an assessment should be performed to establish whether all flow meters are working appropriately

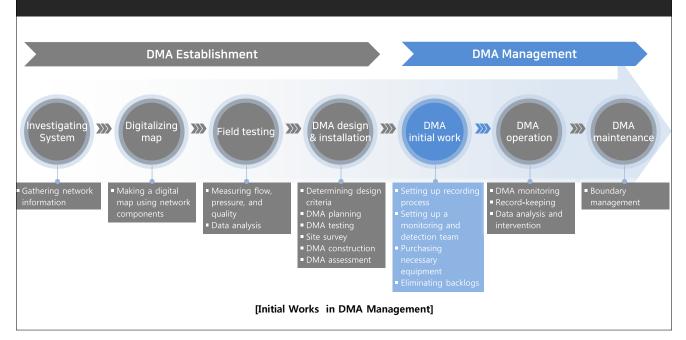
- Inlet flow monitoring: After the successful completion of the initial tests, the total inflow to the DMA should be monitored over several days under normal conditions
- Flow capacity test: After determining that the DMA boundary is hydraulically intact, the operator should confirm that the DMA supply can meet peak demands
 - High flow conditions can be created by opening a boundary valve to a neighboring lower pressure zone or DMA to create an additional flow demand through the subject DMA
 - Alternatively, one or more fire hydrants can be opened to simulate fire-fighting conditions
- Revising the design: If problems were incurred during the tests, then the DMA design should be revised

5. DMA Management

- **5.1 DMA Initial Work**
- 5.2 DMA Operation: DMA Monitoring
- 5.3 DMA Operation: Record-keeping
- 5.4 DMA Operation: Data Analysis and Intervention
- 5.5 DMA Maintenance: Boundary Management

5.1 DMA Initial Work

DMA Initial work: Once the DMA has been set up, initial work should be conducted, such as determining leakage and eliminating the backlogs of detected and undetected leaks



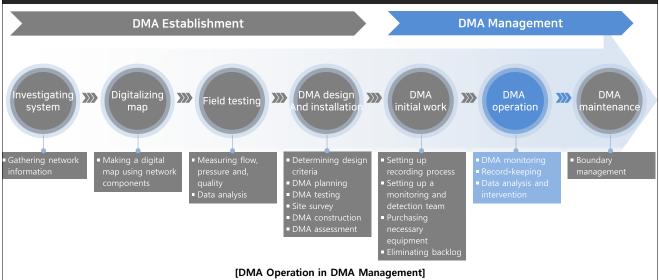
5.1 DMA Initial Work

- Setting up the recording procedures: Data recording forms and procedures for established DMA should be systematized
- Setting up a monitoring and detection team: A team that will take ownership of the monitoring and detection for leakage control should be established
- Purchasing the necessary equipment: Purchasing officers should complete equipment orders as quickly as possible because delays in purchasing will hinder the subsequent process
- Eliminating backlogs: After installing a new DMA, an intensive leak detection and repair campaign should be undertaken to determine and eliminate the backlogs of visible and hidden leaks in the district

5.2 DMA Operation: DMA Monitoring

Monitoring for MNF: Data for the bottom-up approach (i.e., MNF analysis) is possible in cases of sectorized systems and equipped with continuous monitoring

- DMA and night flow analysis are two major tools used in the bottom-up assessment of distribution system leakage



5.3 DMA Operation: Record-keeping

- Record-keeping: Records and files should be collated in a DMA filing system and be accessible to all leakage staff
 - Record-keeping is an essential part of DMA management
- Record types: DMA records should related to physical records (asset records) and record for leakage analysis (operation records)
- Record items: DMA records are static records related to the following:
 - DMA identifiers
 - Meters
 - Boundary valves
 - PRV
 - Pressure monitoring points
 - Mains network and hydrants

5.3 DMA Operation: Record-keeping

Record forms: In addition to PC based-records, each DMA should have a dedicated paper-based file containing all DMA plans and records

- The forms are subject to a company's information system (in-house or bought-in; PC based or paper-based)
- Many companies use GIS to generate plans mounted on portable PCs for field use
- PCs can also contain software for entering DMA data and algorithms for calculating net night flows and leakage



REGIONAL SCHEMATICS

This is a small-scale (e.g., 1:10,000 or 1:25,000) map of the distribution network It should show the DMA information and supply zone drawings



DMA PLANS

Boundary valves and meters, large-metered customers, and features of network and customers with special needs should be included in DMA plans



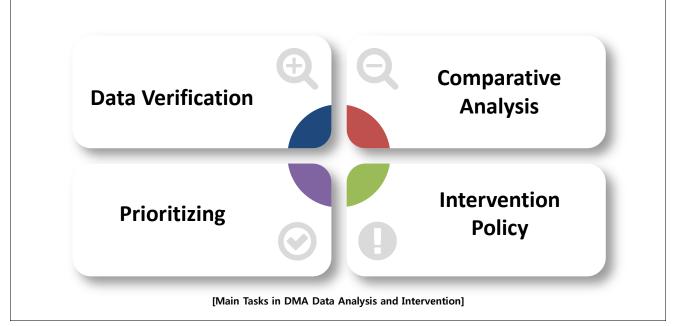
METER RECORDS

These records contain information required for data interpretation and meter maintenance

[Recording Items in DMA Operation]

5.4 DMA Operation: Data Analysis and Intervention

 DMA operation with analysis: The monitoring team analyzes the verified monitoring data to prioritize vulnerable DMAs and implement interventions from the point of view of water loss control



5.4 DMA Operation: Data Analysis and Intervention

(1) Data verification

 Several checks should be conducted by the monitoring the staff before initiating further activities



NIGHT USE ALLOWANCE

Night use allowances should be checked for obvious anomalies



EXCEPTIONAL NIGHT USE

Tracking down exceptional night use is possible, especially if the night use patterns of the large customers are known



REPORTED BURST REPAIRS OUTSTANDING

Any burst repairs in a DMA should be identified by a leakage engineer



VARIATIONS IN NIGHT FLOW Comparison with normal variations in night

Comparison with normal variations in night flow in the DMA and length of time over which the increased flow has been measured

COMPARISON WITH NORMAL



06

BOUNDARY INTEGRITY

Sudden changes in minimum night flow are frequently caused by the closure and opening of DMA boundary valves

CHANGES IN TOTAL DAILY FLOWS

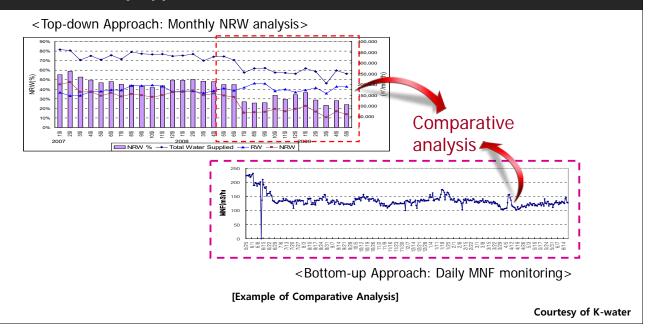
This point should be verified to ensure that the increase in night flow may be related with a corresponding increase in daily flows

[Monitored Data Verification Items]

5.4 DMA Operation: Data Analysis and Intervention

(2) Comparative analysis

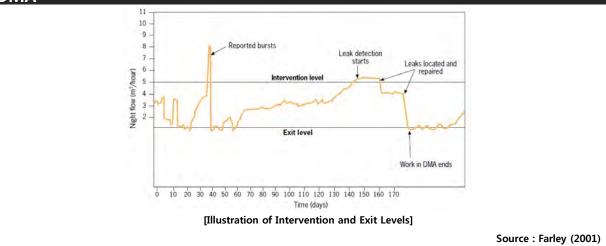
 Before determining the intervention, comparative analysis using top-down and bottom-up approaches should be conducted



5.4 DMA Operation: Data Analysis and Intervention

(3) Intervention policy

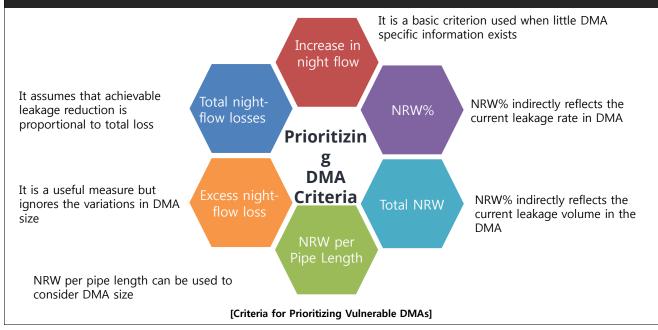
- Setting intervention policies: Intervention denotes the decision-making process for entering a DMA to inspect and locate any leaks and for withdrawing from a DMA
- Establishing the intervention level: Converting leakage targets from the company level to the local intervention and exit levels should be possible in a DMA



5.4 DMA Operation: Data Analysis & Intervention

(4) Priority

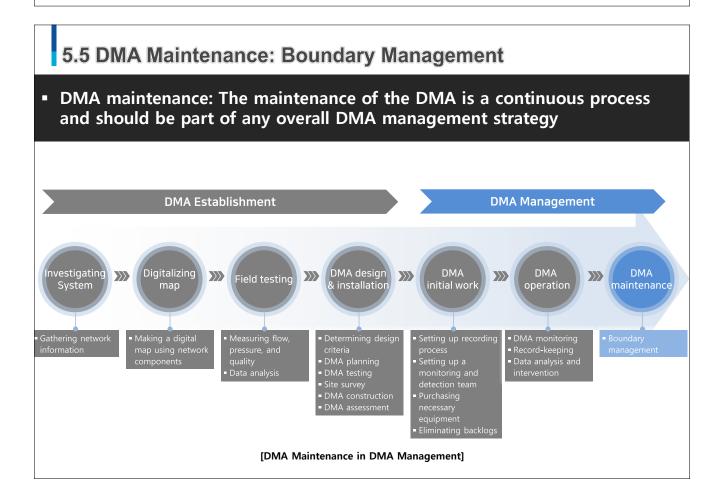
 Prioritizing DMA: Several criteria are in place for prioritizing leakage detection from the results of DMA losses



4 DMA Operation: Data Analysis and Intervention							
DMA	Demand (m¹/day)	Supply (m²/day)	NRW %	NRW (m'/year)	Pipe length (km)	NRW (m³/km)	Priority No. For Activities
Total	24,559	30,617	19.8%	2,211,397	557.411		
NS	7,139	9,885	27.8%	1,002,187	120.166	8,340	3
BH	1,077	1,550	30.5%	172,652	34.362	5,025	5
ND	2,761	3,264	15.4%	183,689	20.592	8,920	2
KG	2,852	3,390	15.9%	196,439	101.995	1,926	7
YM	3,643	4,533	19.6%	324,988	109.453	2,969	6
MS	6,446	6,917	6.8%	172,089	13.604	12,650	1
SD	428	515	17.0%	31,850	88.563	360	8
YS	272	585	53.5%	114,118	22.259	5,127	4

[Example of Prioritizing DMA for Leakage Reduction Activities]

Courtesy of K-water





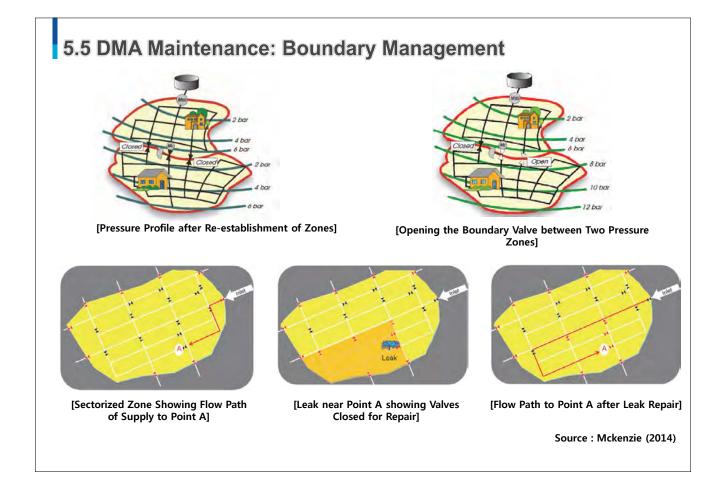
 DMA deterioration: Thus far, the biggest problem facing water utilities worldwide is the continual breaching of DMAs within the system

- Breaching is frequently committed by officials from the water supplier

 No measures for monitoring deterioration: Very few water utilities implement any system in place to monitor and manage their DMAs

- Without such monitoring, the supplier is unaware that the DMA has been compromised and is no longer functioning properly

- Results of DMA deterioration: If any of the boundary valves are opened or closed by mistake (or on purpose), then the DMA can be compromised and will no longer act as a DMA
 - This DMA deterioration leads to a less intact DMA and changes flow path and pressure



5.5 DMA Maintenance: Boundary Management

 Implementing and maintaining routine operation: Utilities always provide budget for ongoing maintenance of DMAs because they must continually be monitored and verified for discreteness at regular intervals

UPDATING KEY

INFORMATION

Information (e.g., DMA boundaries, flow meter locations, and household and other consumers' records) should be up-to-date



REGULAR DISCRETIZATION INSPECTION

Verifying that the DMAs are discrete is part of operation and maintenance



RECORDING OPERATION RECORDS

Records of pipe flushing or boundary valves being opened for operational reasons should be kept

05 Cu

CUSTOMER COMPLAINTS

Customer complaints about low pressure, service interruption, and water quality problems should be monitored to identify potential shortcomings on the part of the DMA



FLOW METERS

Flow meters should be properly maintained to ensure a high level of data accuracy

5.5 DMA Maintenance: Boundary Management

 Protection methods: A number of strategies can be used to address the potential problems caused by unauthorized opening of boundary valves



CUTTING AND CAPPING

The most drastic method is cutting and capping cross-boundary connections



CUT AND INSTALLING FIRE HYDRANTS

This approach denotes cutting pipes and placing fire hydrants on both sides of the zone



COVERING

Covering is to close the valve and fill the box with sand and place a thin layer of cement over the sand



MARKING

Connections between adjacent zones should be clearly marked to ensure that crossboundary connections are closed

[DMA Boundary Protection Methods]



5.5 DMA Maintenance: Boundary Management









(a) Shut-off Valve

(b) Boundary Valve

[Manhole Lid for Different Purposes]



[Approaches Used to Identify Cross-boundary Connections]

Source : Mckenzie (2014)

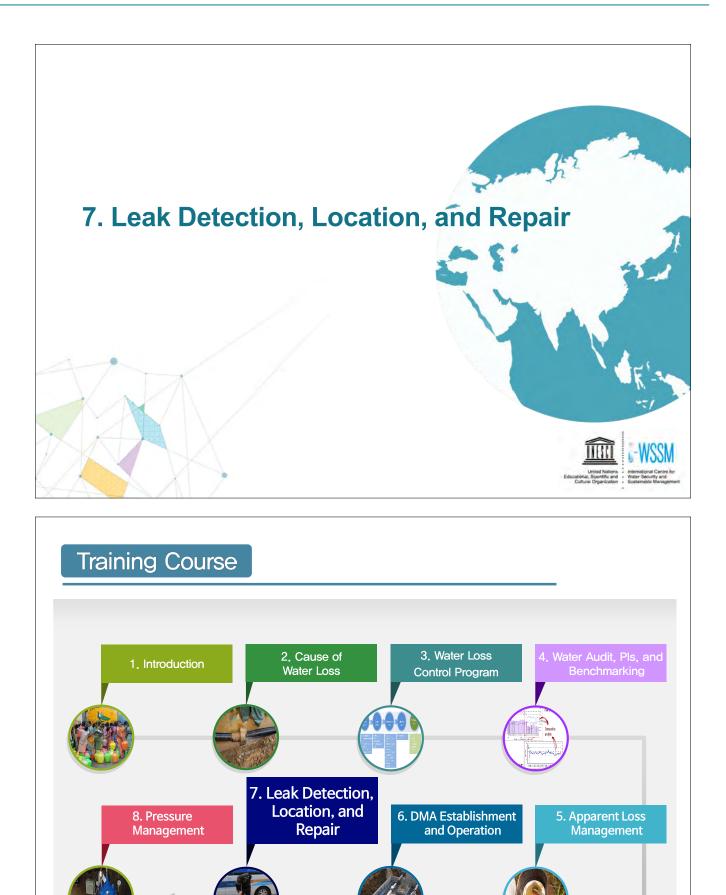
Courtesy of K-water



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	United Nations Educational, Scientific and Cultural Organization	i-WSSM		

Leak Detection, Location, and Repair

Water Loss Management



Aims and Objectives

- The course aims to:
 - Provide knowledge on leakage management procedures including leakage detection and repair methods;
 - (2) Enable trainees to select and implement appropriate leak detection techniques in the water network systems and to repair leakage within a short period
- The objectives are that trainees will understand:
 - (1) The characteristics of acoustic sound;
 - (2) Leakage detection and location equipment
 - (3) Leakage repair methods

Contents

- 1. Overview
- 2. Acoustic Sound Characteristics
- 3. Active and Passive Leakage Control
- 4. Leakage Management Process
- 5. Leakage Awareness
- 6. Leakage Detection
- 7. Leakage Location and Equipment
- 8. Leakage Repair and Replacement

1. Overview 1.1 Background 1.2 Classification of Real Loss 1.3 Causes of Real Loss

1.1 Backgrounds

Terminologies: Leak awareness, detection, and location are defined as follows:

Leak awareness

- Leak awareness denotes a continuous monitoring and analysis of flows to gain awareness of new leaks at an early stage
- DMAs provide a good opportunity for monitoring discrete areas of the water distribution network and facilitate early awareness of even small leaks

Leak detection

- Leak detection pertains to the narrowing down of a leak or leaks to a section of the pipe network
- Leak detection activities may be carried out routinely or in precise areas of the network and guided by analysis of DMA data

Leak location

- Leak location is the identification of the position of a leak prior to excavation and repair, although finding the exact location cannot be guaranteed
- Location survey can be carried out with or without prior leak detection activities

1.1 Backgrounds

- Fundamental activity: Leakage control and management is the most <u>fundamental activity in reducing real loss</u>
 - Efficiently undertaking and managing leak repairs is a pre-requisite for effective leakage management and control
- General activity: Identifying and repairing leaks are considered in addressing water loss
- Initial activity: An intensive program for leak detection and repairs can be an important as an early step when funding for a comprehensive NRW program is initially lacking

1.1 Backgrounds

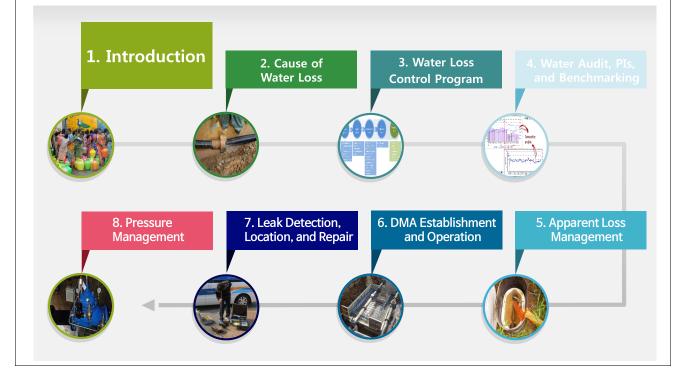
- Long-term activity: Leak detection should not be regarded as a one-time exercise
 - Once initial leak repairs have been undertaken, the area should be re-tested to ensure that large leaks have been identified
- Increasing difficulties: The diversity of new materials used in networks, especially with plastic mains, combined with low operating pressure and lack of access points, have limited the application of acoustic techniques
 - Leak detection is becoming increasingly difficult due to the greater use of plastic pipes, which do not transmit noise from the leak in the same extent as metal pipes
- Difficulty in water supply: In systems with poor pressure and intermittent supply, leakage and burst rates are reduced but identifying leak location is more difficult

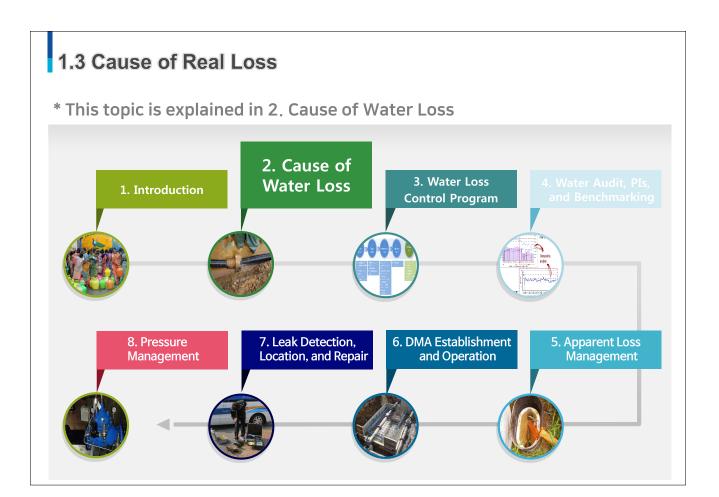
1.1 Backgrounds

- Balloon effect: Notably, fixing leaks will result in an increase in the system pressure which, in turn, will push up the leakage through any existing leaks and cause the formation of new leaks
 - Therefore, any large-scale leak location and repair projects should be undertaken with a form of pressure management, where the system pressures can be reduced if necessary as the leaks are repaired
 - If the pressure is not managed properly, then system leakage will quickly revert to pre-leak repair levels and will be no longer beneficial
- Economics: Assembling a cost-effective basis is important for the definition of the size, schedule, and functions of the leak detection program
 - The costs of creating an in-house leak detection staff or contracting leak detection services can be considerable

1.2 Classification of Real Loss

* This topic was explained in 1. Introduction





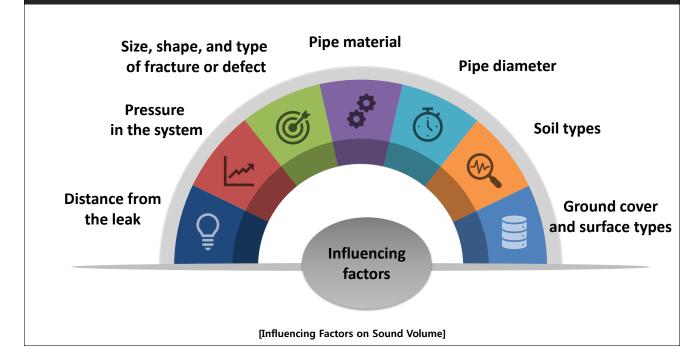


2.1 Sound

- Definition: Sound is defined as oscillation in pressure, stress, particle displacement, particle velocity, and other aspects propagated in a medium with internal forces (e.g., elastic or viscous) or the superposition of such propagated oscillation
- Sound generation: When water escapes from a pressurized pipeline, a characteristic and recognizable leak sound is generated in the pipe at the point of leakage
 - Pressurized water force through a leak loses energy to the pipe wall (structure-born noise) and to the surrounding soil area (soil-born noise)
 - The energy creates audible sound waves that can be sensed and amplified by an electronic transducer or, in some cases, simple mechanical devices
- Sound transmission: Vibration is transmitted along the pipe as structureborne and in the surrounding underground as ground-borne noise

2.2 Volume

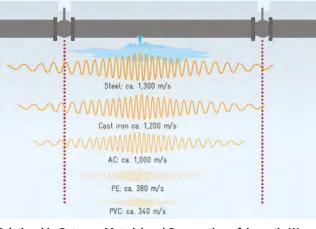
 Volume of sound: In general, the volume of the noise will be dependent on the following factors:



2.3 Speed

Wave speed: The noise is distributed along the pipe at a speed dependent on the particular characteristics of the material

- Air: 340 m/s
- Water: 1,493 m/s
- Aluminum: 5,100 m/s
- Water pipe: approximately 1,000 m/s



[Relationship Between Material and Propagation of Acoustic Waves]

Source: GIZ (2011)

2.4 Travel Distance

 Travel distance: On metal pipes, noise can sometimes be heard from over 100 m. However, on plastic pipes, noise may disappear within a few meters and cannot be picked up unless the observer is on top of the leak

Type of Pipe	Pipe Dia.	Typical Sound Travel Distance		
	6"	1,000 – 1,200 ft		
Iron Pipe	12"	800 – 1,000 ft		
	24"	600 – 800 ft		
C. A. T. C.	6"	800 – 1000 ft		
AC Pipe	12"	700 – 900 ft		
1000	24"	400 – 600 ft		
Sec. 1	6"	400 – 600 ft		
PVC Pipe	12"	200 – 300 ft		
	24"	100 – 150 ft		
Leak N		stances in <u>Service</u> Lines leak @ 50 psi)		
Copper Tubing		600 – 1,000 ft		
Galvanized Steel Pipe		800 - 1,200 ft		
"Poly" Plastic Tubing		50 – 100 ft		

[Leak Noise Travel for Distances in Distribution Mains and Service Lines]

Source: EPA (2011)

2.5 Frequency

Frequency: Vibration can be perceived at different frequencies and heard through a device similar in operation to an amplified stethoscope

- Vibrations are frequently detected on the ground surface in the 200 Hz to 600 Hz frequency range and directly on the distribution network mains between 600 Hz and 2,000 Hz

Types of leak sounds	Details
Orifice-pipe vibration	 Leak noises within a range of 500 Hz to 800 Hz usually originate as orifice-pipe vibration phenomenon and are transmitted along the pipe wall and in the water and in many instances, a considerable distance from the actual leak Identifying this sound by systematically testing valves, hydrants, and curb-stop valves frequently locates potential leaks
Impact of water on soil	 This leak sound emanates from a range of 20 Hz to 250 Hz It is caused by the impact of water on soil in the area of the leak In contrast to vibration on the pipe wall, the travel distance of the lower frequency sounds is limited to the immediate area of the leak Due to the limited range, these low-frequency sounds are essential for pinpointing the leak
Water circulation	 This leak sound emanates from a range of 20 Hz to 250 Hz The sound is caused by water circulation, usually in a cavity in the soil near the leak The sound resembles those of water emanating from a fountain In contrast to the vibration on the pipe wall, the travel distance of the low-frequency sounds is limited to the immediate area of the leak Due to limited range, these low-frequency sounds are essential for pinpointing the leak



3.1 Leakage Control Policy

- 3.2 Passive Leakage Control (PLC)
- 3.3 Active Leakage Control (ALC)

3.1 Leakage Control Policy

- Leakage control policy: As part of the strategy implementation, the policy aspect of leakage management can be classified into two operational policies as follows.
 - Passive leakage control (PLC)
 - Active (proactive) leakage control (ALC)
- Selecting a policy: The choice of policy to be taken by the utility is dependent intrinsically on the systems and financial capacity as well as the determinants of servicelevel aspects, such as network pressure, burst frequency, level of background leakage, leak identification, and location times as as well as the stipulated time for repair

3.2 Passive Leakage Control

 PLC: pertains to reacting to reported bursts or drops in pressure, which are typically reported by customers or noted by the utility staff while implementing duties apart from leak detection

No maintenance

Event-driven maintena

Interval-oriented mainten

Condition-oriented maintena

vel of real losses 🚿 Burs

[Comparison Between Different Intervention Strategies to Combat Real Loss]

Source: GIZ (2011)

- Usually practiced in less developed systems: This type of leakage control is practiced in less developed supply systems, where the occurrence of underground leakage is less understood
- High leakage: Overall losses are typically high because no efforts are made to locate and repair unreported leakage and reduce background leakage
- Increasing leakage: Without exceptional circumstances, leakage will continue to increase under PLC

3.2 Passive Leakage Control

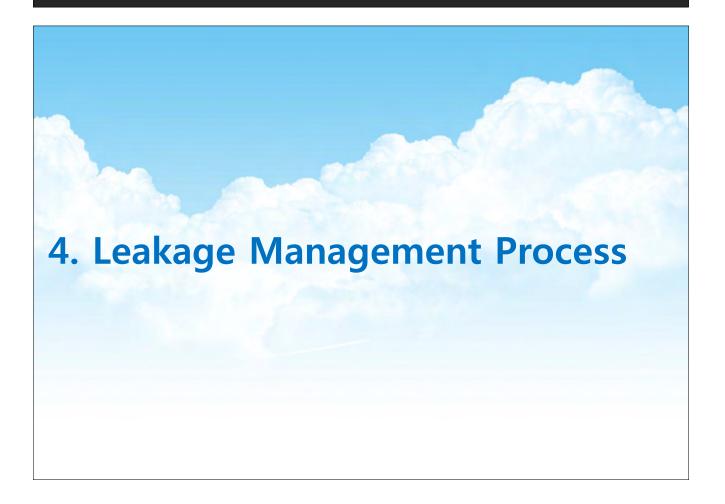
- Failure to reach the desirable level: Notably, the PLC cannot, in general, attain desirable levels of efficiency, particularly in terms of water loss
- Cost effectiveness: Although PLC is clearly not ideal from the viewpoint of leakage reduction, the key issue is determining which is cost effective in PLC and ALC
 - PLC can be effective for a limited period of time in new systems built from scratch using appropriate and quality projects and materials
- Appropriate situations: PLC can be justified in areas with adequate raw water resources and water treatment capacity and low marginal costs

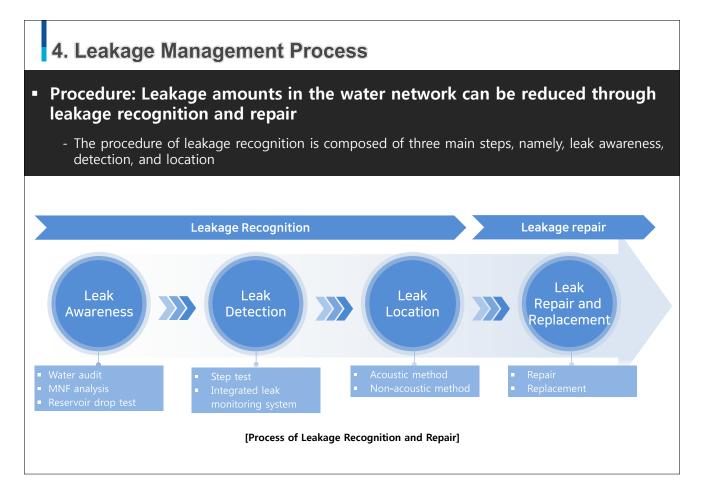
3.3 Active Leakage Control

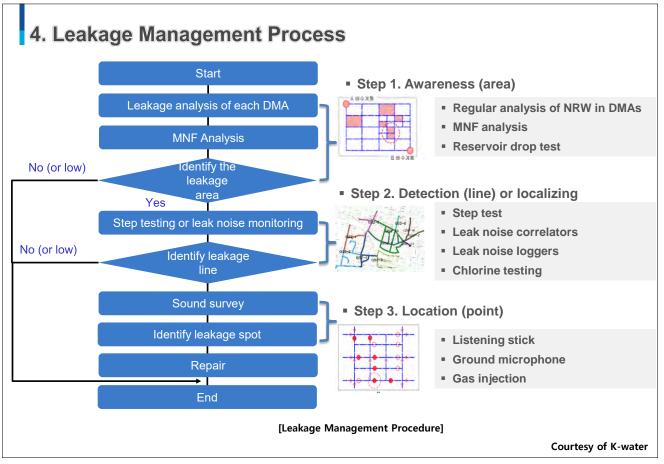
- ALC: An intervention method for counteracting real loss. A water utility deploys funds, personnel, and technical equipment to actively detect and repair unreported leaks
- Repair visible leak first: Notably, all visible leaks should be repaired before any activities are undertaken to locate new and unreported leaks
 - If the initial visual inspection highlights a large number of clearly visible leaks, which are running without being repaired, then efforts should be exerted to locate leaks that have not come to the surface only after the obvious leaks have been eliminated
- ALC with quick repair: Undertaking ALC does not solve the problem. In all cases, the utility must continue to ensure that all reported or detected leaks are repaired quickly and effectively
 - A utility with a proper and efficient PLC process in place can drive leakage down to acceptable levels, whereas a utility undertaking ALC on a regular basis but does not repair the leaks quickly or properly may face a serious leakage problem

3.3 Active Leakage Control

- Combined with awareness: ALC is cost effective in certain cases but should not be undertaken in a blanket fashion but rather in a targeted fashion, where specific problem areas are firstly identified (e.g., MNF analysis)
- Cost effectiveness: ALC may or may not be cost effective depending upon the level of leakage in a specific area
 - If an area is known to have high leakage (e.g., the area will have high MNF), and the network is known to be in a poor condition, then sending in a team of leak locators to identify unreported leaks may be worthwhile and cost effective



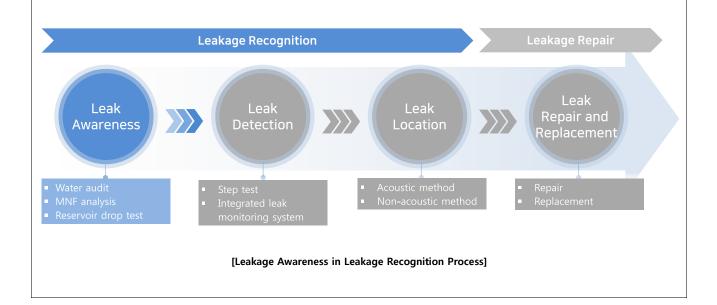






5.1 Leakage Awareness

 Leak awareness: Modern flow metering and data capture technologies play a major part in the rapid identification of bursts and estimating the gradual accumulation of small leaks



5.1 Leak Awareness

- Methods: In the DMA inlet, flow and pressure can be monitored and analyzed
- DMÁs and monitoring system: To effectively monitor flow and pressure, adequate DMAs and DMA inlet monitoring system should be established

FLOW MONITORING

- Pipe breaks and bursts cause a more or less abrupt rise in the flow rate, which can be detected by constantly monitoring the inflow into an open network or DMA
- To this end, inflow should be monitored continuously or for at least 1 h during MNF conditions and compared to previously measured reference values



PRESSURE MONITORING

- Monitoring pressure in a network can only detect major leaks because small leakage flow rates will not significantly reduce pressure

[Methods for Leakage Awareness]

5.1 Leak Awareness



(a) Flow and Pressure Meter





(c) SCADA System

(b) PLC or RTU



(d) Monitoring and Analysis

[DMA Inlet Monitoring System]

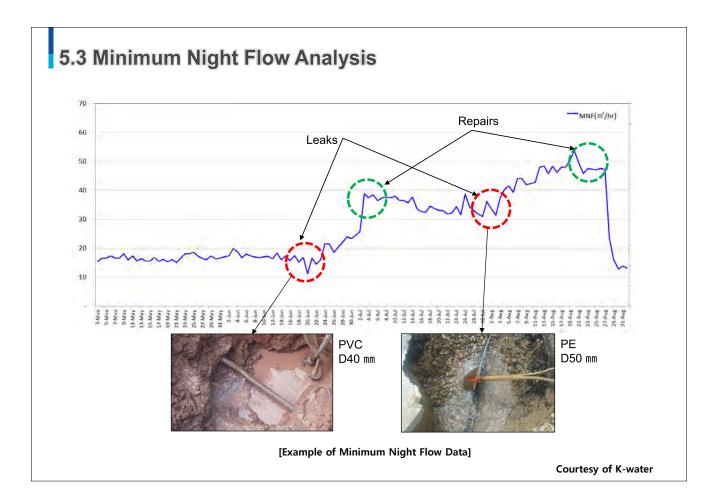
5.2 Minimum Night Flow Analysis

 MNF analysis: A gradual increase in minimum hour flows over periods of days or weeks is a good indication that new leaks have developed

	Initial DMA performance	4	- Total & Nightline volume;	Ľ			
li	indicator analysis DMA ranking of intervention priority	÷	- % Minimum-to-average flow Definition of target nightline & recoverable losses	k	 Assessment of minimum; Authorised consumption (mor & unavoidable leakage (UAR) 		stimated)
1			Selection of leak detection techniques	+	Boundary valve verification		
			NO	→	Leak location	+	Communication of repair request
							*
L		YES	TARGET NIGHTLINE ACHIEVED?	_		_	Leak repair

5.2 Minimum Night Flow Analysis

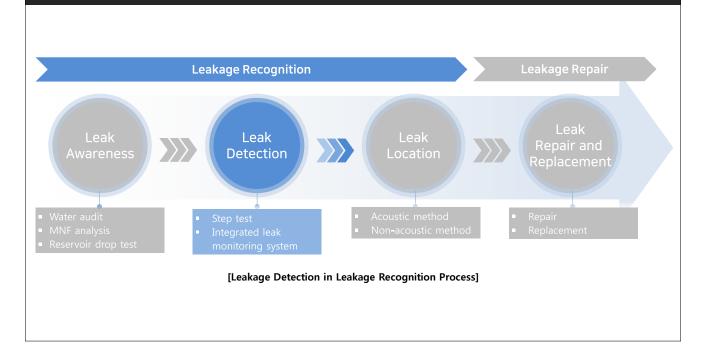
- Without DMA: If the DMA system is not constructed, then leakage analysis can be performed per unit of service reservoirs
- Monitoring large consumers: Large-volume night users (e.g., factories) should have their meters read manually during the testing period if logging is unavailable
- Average zone night pressure: The average zone night pressure should be recorded within the same period
 - These data will not be used for MNF analysis but will provide valuable data for pressure reduction purposes and future sophisticated leakage calculations
- Waiting a few days: If the increased flow rate remains stable for more than three days, then the change is probably caused by a leakage and not exceptional customer usage





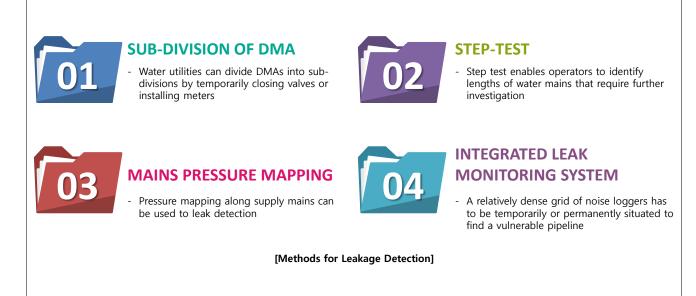
6.1 Leakage Detection

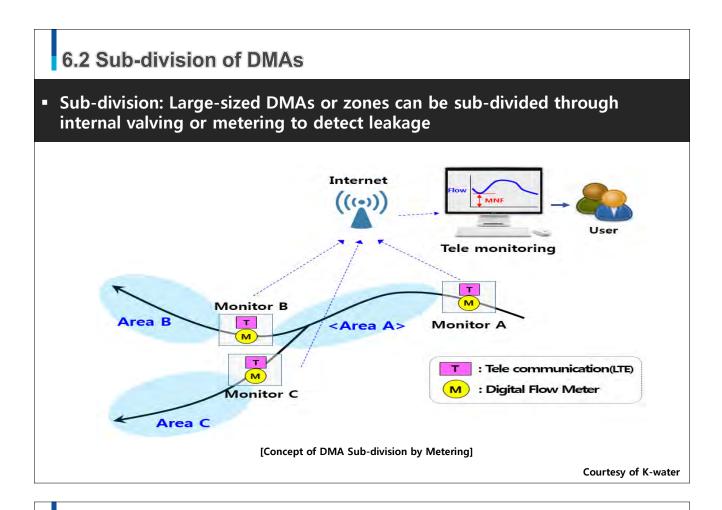
 Procedure: For suspicious DMAs, leak detection is performed based on the main pipeline



6.1 Leakage Detection

 Methods: To narrow down the leak area, sub-division of DMA, step-test, main pressure mapping, and integrated leak monitoring system can be implemented



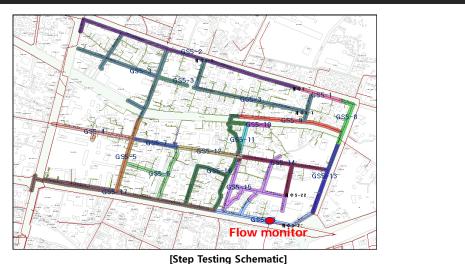


- 6.2 Sub-division of DMAs
- Targets: The sub-zoning technique is applied when the identification of excessive consumption in a DMA is considered
- Implementation with smart meters: With the increased use of smart meters, this method has become widely adopted in places where DMAs are not established
- Checking conditions: A procedure for checking conditions prior to the implementation of this type of test should be created to guarantee minimum pressure and water quality
 - Notably, the opening of DMA boundary valves can cause changes in water quality if a length of pipe exists without considerable regular consumption upstream of these valves
 - In these cases, discharge using a specific valve or fire hydrant should be performed beforehand to mitigate potential problems with water quality

6.3 Step Test

 Principle: The principle of the technique is to systematically reduce the size of the area by closing valves on each section of a pipe and noting changes in flow rate at the meter at the same time.

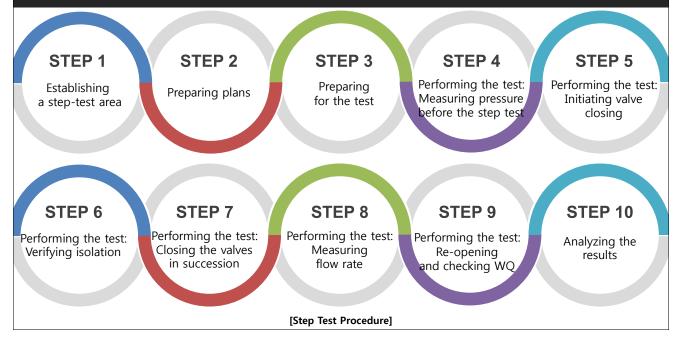
- Step testing should be conducted at night when demand is at the minimum level, and consumers should be notified beforehand that the supply will be disrupted



Courtesy of K-water

6.3 Step Test

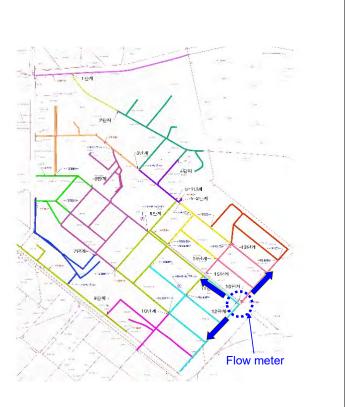
 Procedure: Step testing should be conducted at night when demand is at the minimum level, and customers should be notified beforehand that the supply will be disrupted



6.3 Step Test

Test Overview

- Method: Complete Isolation
- Steps: 16 steps (valve: 33)
- Flow meter: Electromagnetic (m³/min, m³/h)
- Pressure: 5.0 kgf/cm² (fixed)
- Water supply shut-off : 800 households
- Date/time: 23 March/01 a.m.-05 a.m.



[Establishing a Step-test Area and Preparing Plans]

Courtesy of K-water

6.3 Step Test





[Preparation for the Test: Boundary Valve Check and Closure]

Courtesy of K-water





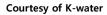
[Preparation for the Test: Step Test Valve Test]

6.3 Step Test





[Preparation for the Test: Investigating Pressure Check Points]

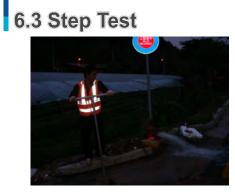


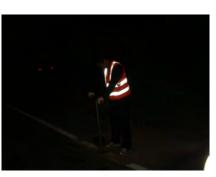




[Performing the Test: Measuring Pressure Before Step Test]

Courtesy of K-water





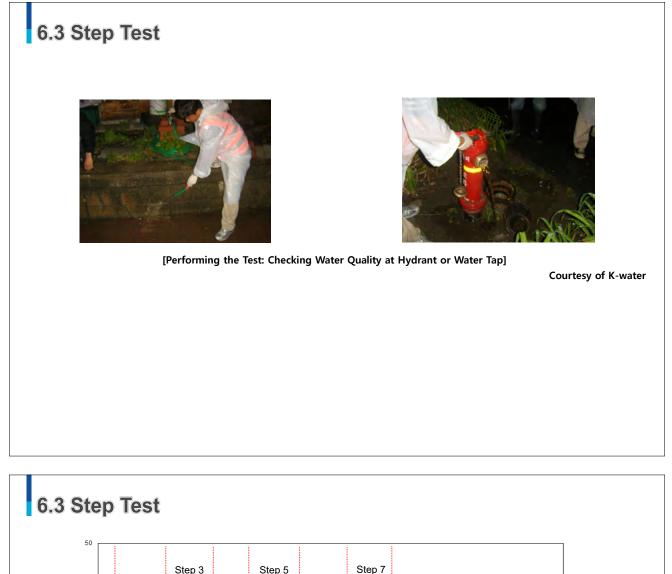
[Performing the Test: Initiating the Step Test (Valve Closure)]

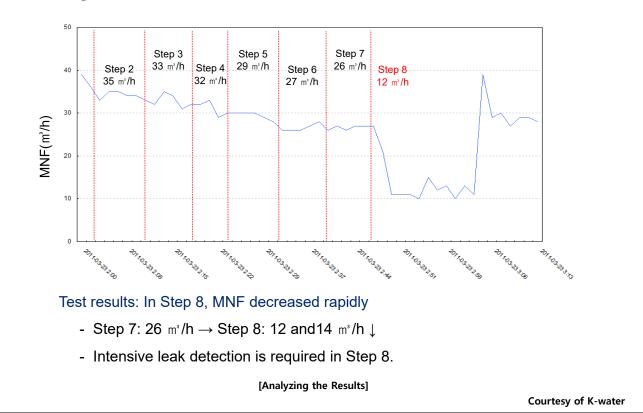
Courtesy of K-water



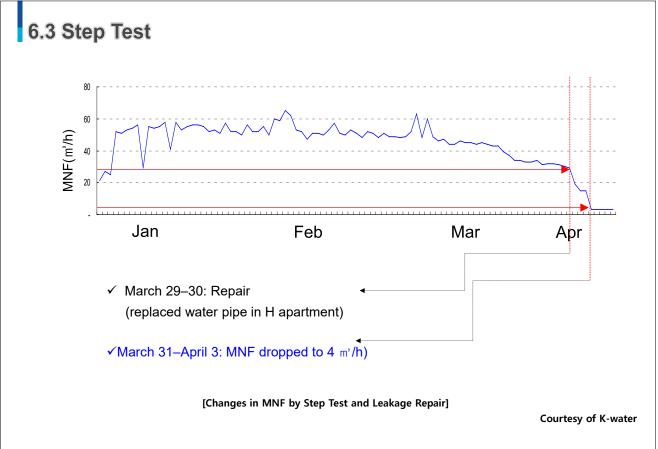


[Performing the Test: Valve Re-opening]









6.4 Integrated Leak Monitoring System

 Objectives: Leak noise monitoring system aims for automatic monitoring and data collection with cost-effective, wireless, and movable noise loggers



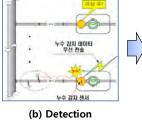


[Installing a Noise Logger on a Valve or Meter]



Courtesy of USOL







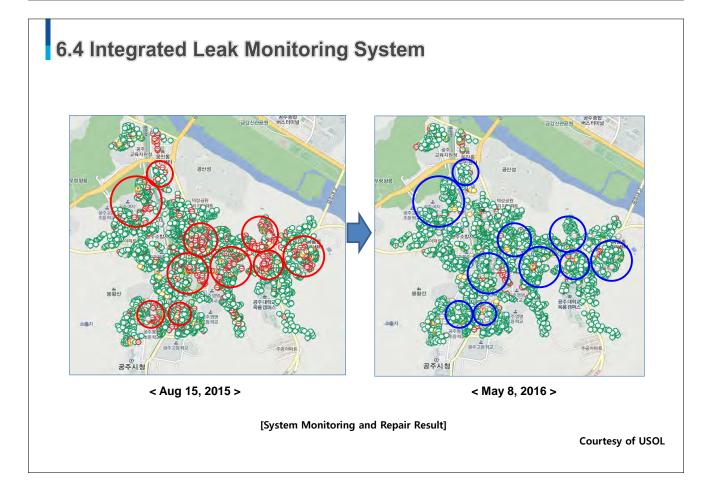
(c) Monitoring

[Operating Process]



(d) Pinpointing and Repair

Courtesy of USOL

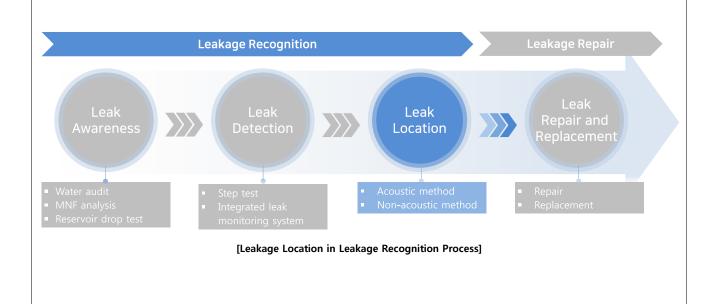


7. Leakage Location and Equipment

- 7.1 Leakage Location
- 7.2 Leak Survey Program
- 7.3 Equipment Types and Selection
- 7.3 Acoustic Equipment
- 7.4 Non-acoustic Equipment

7.1 Leakage Location

 Leak location: Once the approximate area of a leak has been determined, leak location methods should be used to determine the exact location (±1 m) to reduce excavation efforts

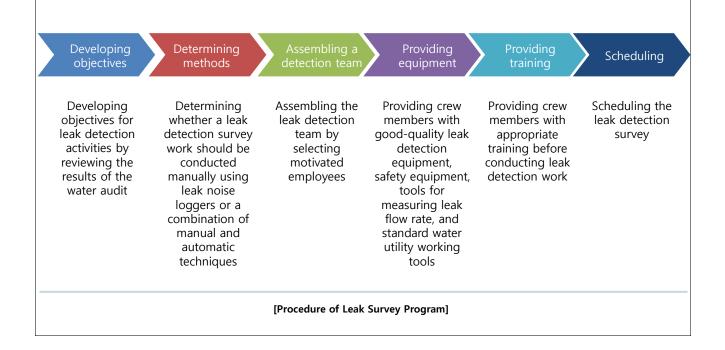


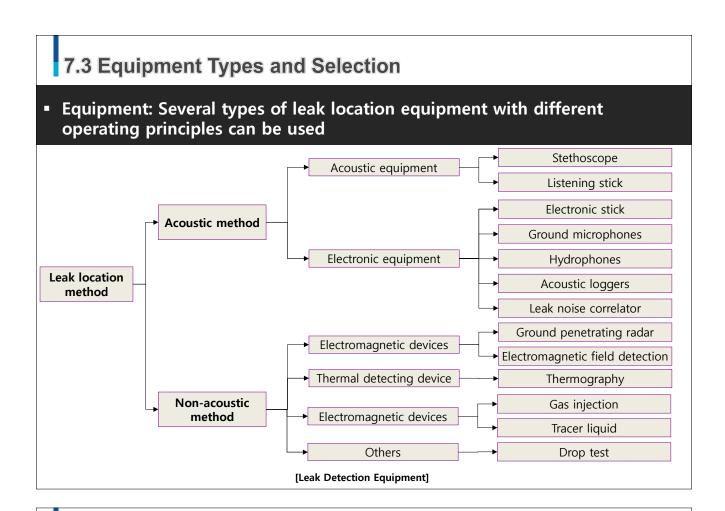
7.1 Leakage Location

- Acoustic methods: Spotting the leakage points are performed by an acoustic detection check on valves, hydrants, and ground surface
 - Several acoustic methods can be performed, such as listening sticks, ground microphones, and leak noise correlation
 - If acoustic methods are unsuccessful, then several non-acoustic methods are available
- Must be performed after leak detection: Interventions must be preceded by an analysis process to identify vulnerable pipelines in the distribution network to reduce the time-intensive and labor-consumption leak location process

7.2 Leak Survey Program

 Leak survey program: The procedure in creating an in-house leak localizing program is as follows:





7.3 Equipment Types and Selection

 Selecting the equipment: Given the inherent supply conditions, the utilities should examine the network characteristics and the experience and ability of operators when selecting appropriate equipment

Pipe material	Small-diameter pipes (75–400 mm)	Large-diameter pipes (from 300 mm)
Metals	 Gas injection Manual listening stick Accelerometer Hydrophone Leak noise loggers 	AccelerometerHydrophonesIn-line detection techniques
AC	Gas injectionAccelerometerHydrophone	- In-line detection techniques
PVC	- Gas injection - Hydrophone	- In-line detection techniques
Polyethylene	- Gas injection - Hydrophone	- In-line detection techniques

7.3 Equipment Types and Selection

- Big market: ALC is a multi-billion dollar industry worldwide with many large companies specializing in developing and manufacturing various types of equipment for detecting leaks
- Exaggeration of equipment: Various manufacturers claim their equipment is a silver bullet for leak location and will find all leaks within a particular supply system
- Expensive equipment not required: The equipment used to identify unreported leaks should not be the most expensive or most sophisticated
 - In many cases, a well-trained experience leak detector with a basic listening stick will frequently find more leaks than a poorly-trained leek locator with the most expensive equipment

7.4 Acoustic Equipment

- Acoustic methods are based on the fact that a leak in a pipeline under pressure emits a permanent specific noise and is defined by a particular range of frequencies
 - This method is the basic one used for leak location, which uses the human ear (with a listening stick or stethoscope) or electronic listening devices, such as ground microphones and leak noise correlators
- Electronic devices have amplifiers, feature-insulated headphones, filters to screen out selected frequencies, and readout devices to provide a visual measure of noise
 - Electronic listening sticks come in various levels of sophistication. However, they are basically listening devices with an electronic amplification of the sound generated by leaks
- Conducting without interruption: One of the main advantages of applying this technique is the possibility of undertaking the pinpointing work without the need for supply suspension

(1) Listening stick or stethoscope

- A stethoscope or listening stick with an earpiece is pressed against fittings to listen for nearby leaks in the pipe
- Affordable equipment: The basic listening stick or pocket stethoscope is an inexpensive tool that is effective and requires no power or maintenance
- Training: The uytility must provide proper training to the staff to maximize equipment use



[Listening Stick]

Courtesy of K-water

7.4 Acoustic Equipment

(2) Microphone: Electronic sticks

 Electronic listening sticks come in various levels of sophistication. However, they are basic listening devices that electronically amplifies the sound generated by the leak



[Use of Electronic Listening Stick]

Source : EPAL (2017)

(3) Microphone: Ground microphone or geophone

• When the acoustic method is applied to listen to the ground surface above the mains, then a geophone or ground microphone is utilized



[Ground Microphone]

Courtesy of K-water

7.4 Acoustic Equipment

(4) Leak noise logger (acoustic loggers)

- Principle: Analysis of the readings is conducted by comparing the sound level and sound spread recorded at each logger
 - The reading indicates the existence of leakage in the vicinity around the installation site, which is considered suspicious and, thus, subject to a detailed inspection using another type of leak detection equipment

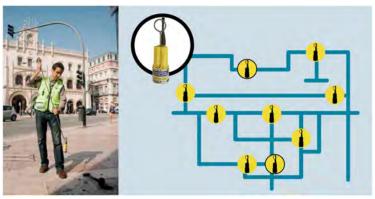


[Acoustic Logger #1] Source : Trent (2005)



[Acoustic Logger #2] Source: http://www.seoyong.co.kr

 Installation: In planning the installation process, two important aspects should be considered, namely, the material of the distribution network mains and existing pressure service because these two factors largely influence noise propagation

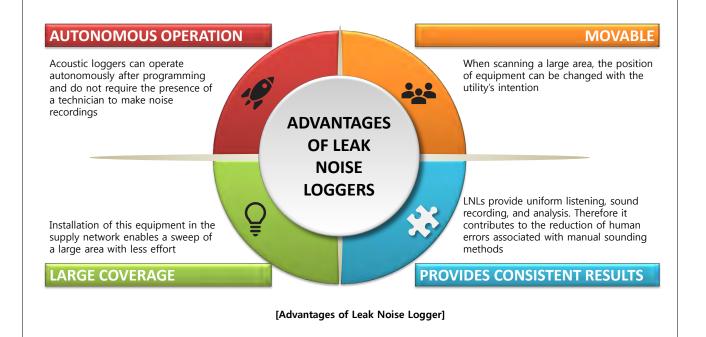


[Installation of Acoustic Noise Loggers]

Source : EPAL (2017)

7.4 Acoustic Equipment

Advantages: Leak noise loggers have several advantages as follows



- (5) Leak noise correlators (LNCs)
- Principle: LNCs compare noise detected at two points on the pipeline

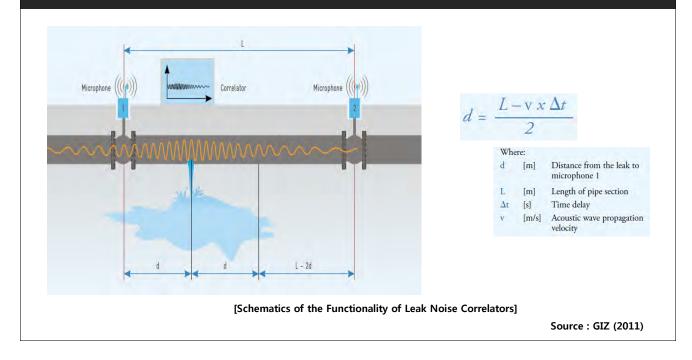


[Leak Noise Correlator]

Courtesy of K-water

7.4 Acoustic Equipment

 Calculation: Correlation is a mathematical method for calculating the time delay between two signs emitted by the same source



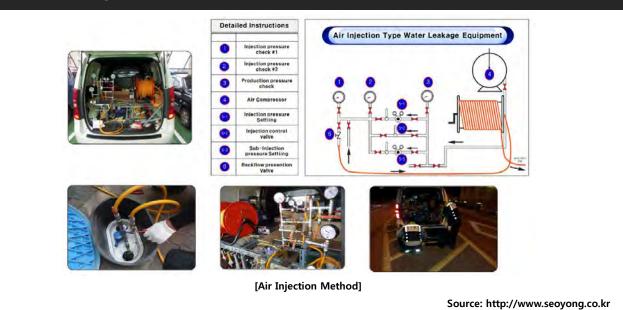
7.5 Non-acoustic Equipment

- A good alternative to acoustic methods: Non-acoustic methods may be a viable alternative to acoustic methods in networks with very low pressure or intermittent supply or for locating hard-to-find background leakage
- Requires training: Non-acoustic leak location methods usually require specially skilled personnel and involve great efforts
- Last resort: Non-acoustic method is very effective. However, it is expensive. Thus, this method should be applied only as a last resort

7.5 Non-acoustic Equipment

(1) Gas injection

• Principle: A tracer gas is injected and subsequently detected on the surface above the alignment of the mains



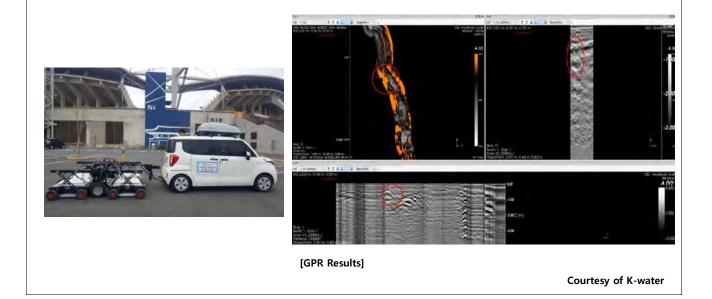
7.5 Non-acoustic Equipment

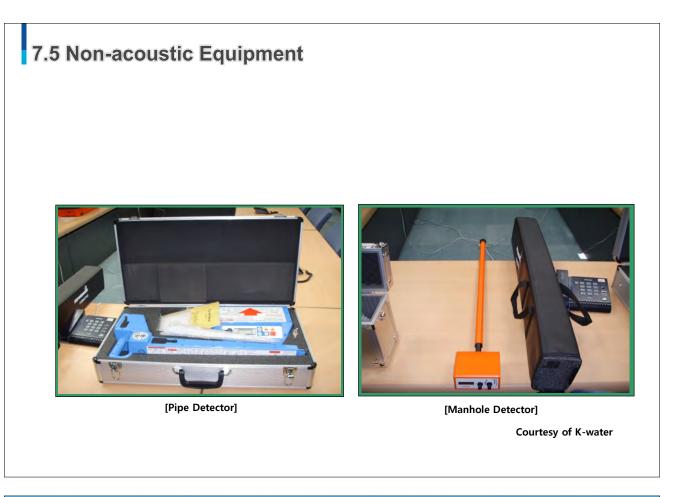
- Process: The process starts with a supply suspension of the test section as well as all service connections prior to injecting the tracer gas through a hydrant or a similar accessory with sufficient pressure. In this manner, the gas is released through the leak hole in the mains
- Pros and cons: This method is very effective but costly because it involves consumables, such as the tracer gas
- Gas: Hydrogen is the most common gas employed, but helium can also be used

7.5 Non-acoustic Equipment

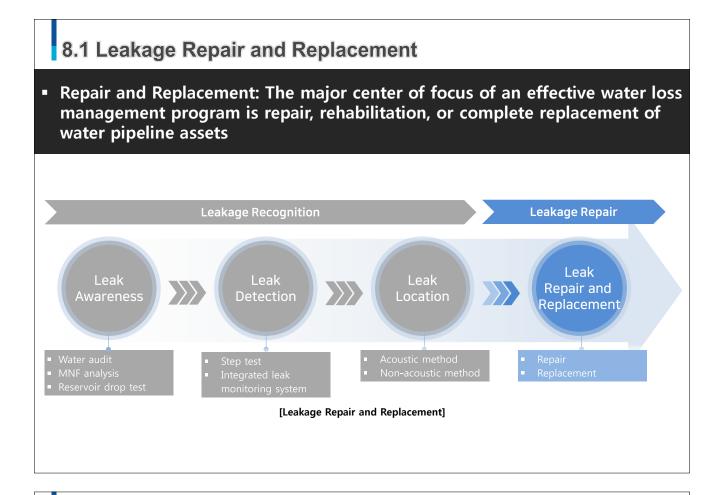
(2) Ground Penetrating Radar

 Principle: The ground penetrating radar operates on the principle of emitting a radar pulse through an antenna into the ground









8.1 Leakage Repair and Replacement

 Repair quality: The quality of leak repairs plays a significant role in the overall leakage management process

- Poor materials, need for improvisation, and bad workmanship can lead to the probability that that new leaks will occur in the same place
- Therefore, leak repairs should be executed by well-trained and skilled laborers
- Despite the cost of repairing a leak, fixing it for the second time can be more than double the investment in labor and materials and leads to customer dissatisfaction
- Most expensive step in repair: Many studies demonstrated that the most significant portion of leak repair cost and time is attributed to uncovering the leak sites and dewatering

8.1 Leakage Repair and Replacement

- Good personnel (well-motivated and equipped): A leak repair personnel should be well motivated and equipped with the necessary equipment, vehicles, communication devices, and safety gear
- Good personnel (well-trained): Repair crews should be trained on a variety of fix approaches, such as a repair clamp, in-situ welding, or replacement of one or more sections of a pipe
- Good personnel (sufficient crews): The number of repair crews should be appropriate for the number of leaks and bursts to be repaired in specified target times



[Training: Competition for Leakage Detection] Courtesy of K-water



[Training: Competition for Leakage Repair] Source: Water Korea (2007)

8.1 Leakage Repair and Replacement

 Stock management: The warehousing department should ensure that frequently used parts are in stock because long delivery times for required spare parts are not tolerable



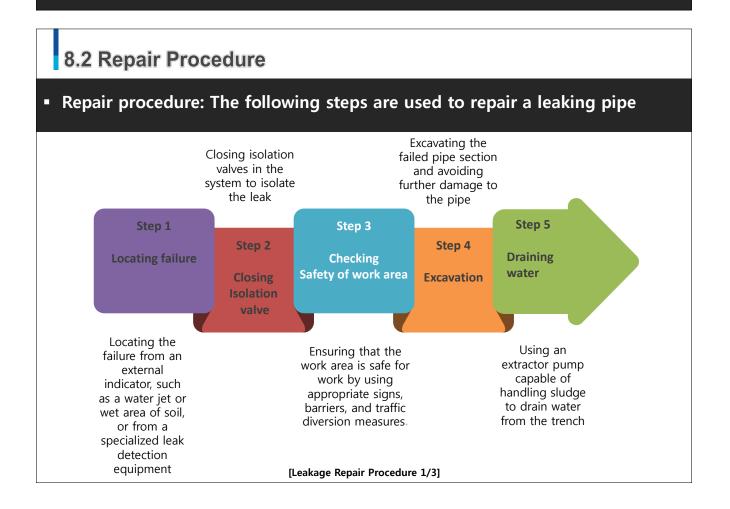
[Warehouse #1]

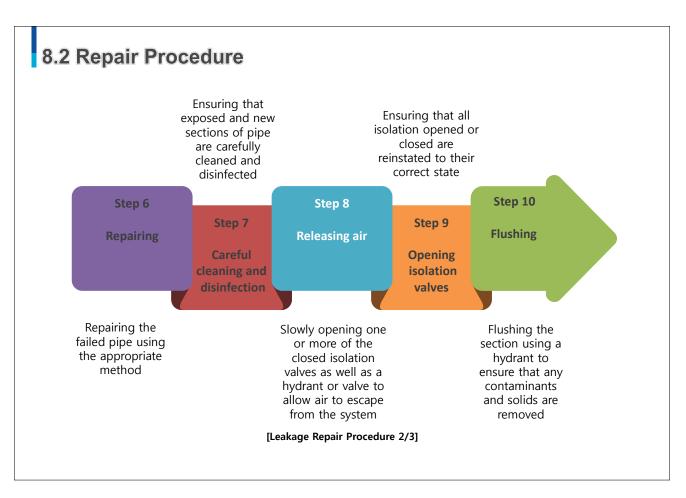


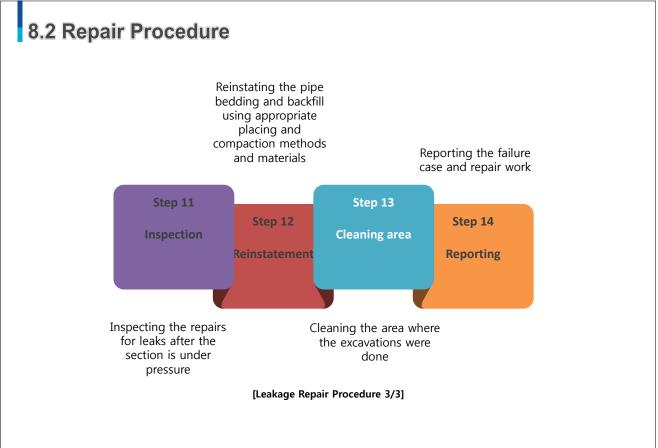
[Warehouse #2]

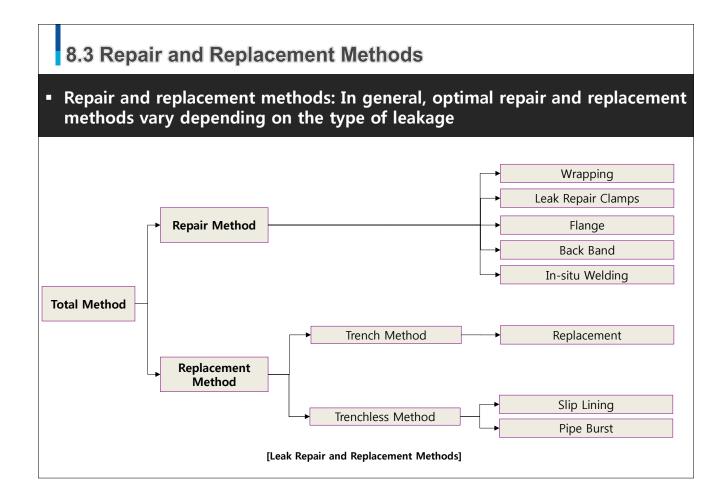
8.1 Leakage Repair and Replacement

- Analysis of failure DB: Analyzing the failure database and failure rates may identify damage-prone types of pipes and valves
 - Analysis of the leak repair history of the utility can greatly facilitate the selection of appropriate materials and quantities to stock
- Networking: Many small-sized water utilities found that reaching out to neighboring water utilities that may have similar repair parts and equipment is advantageous for emergency cases
- Policy (effectiveness): The water utility's policy exert a strong influence on the speed and quality of repairs
- Policy (increasing performance): Repair times can be reduced by setting performance targets for how fast a main break, a service connection leak, or a detective valve can be repaired (e.g., all leaks found should be repaired within 24 h)









8.3 Repair & Replacement Methods

(1) Repair: Wrapping

 Wrapping: Small pipe leak repairs may be made using a surface wrap depending on the pipe material



[Wrapping]

Source : EPA (2010)

8.3 Repair & Replacement Methods

(2) Repair: Leak repair clamps (flexible clamps), flange, backing band, among others

• Leak repair clamp: If the leak site is the result of small corrosion pitting or puncture holes, then a repair clamp will usually work quickly and well





[Pipe Repair Using Repair Clamp #1]

Courtesy of K-water

8.3 Repair and Replacement Methods



[Pipe Repair Using Repair Clamp #2]

8.3 Repair and Replacement Methods



[Pipe Repair Using a Mechanical Joint (확인필요)]

Courtesy of K-water

8.3 Repair and Replacement Methods



[Pipe Failure and Repair by External Force During Road Construction #1]



8.3 Repair and Replacement Methods

(3) Repair: In-situ welding

• For large steel pipes, repair may be in the form of in-situ welding





[Pipe Failure and Repair by External Force]



Courtesy of K-water

8.3 Repair and Replacement Methods

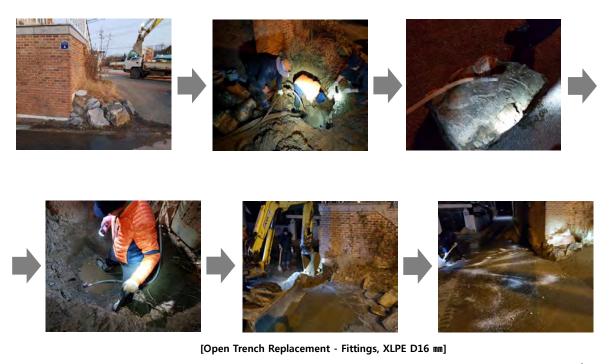
(4) Replacement: Open trench replacement

 Repair crews typically use this method to discover if a section of a leaking pipe is extremely deteriorated for repair with the application of a simple repair clamp



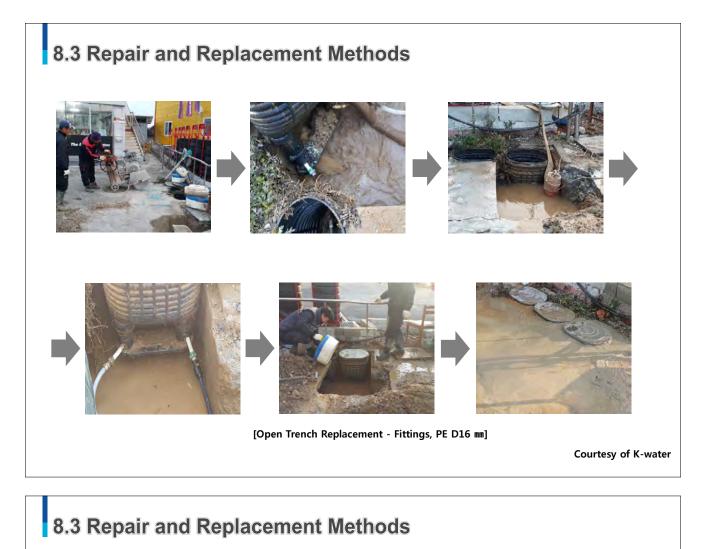
[Open Trench Method]

8.3 Repair and Replacement Methods



Courtesy of K-water

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8.3 Repair and Replacement Methods

(5) Replacement: Slip lining as a trenchless replacement

 Slip lining is an effective approach for rehabilitating a deteriorating water main without uncovering it



[Slip Lining]

Courtesy of K-water

8.3 Repair and Replacement Methods

(6) Replacement: Pipe bursting as a trenchless replacement

 Pipe bursting is an alternative approach to destroying the existing pipe as a new one is being dragged through it



[Pipe Bursting]

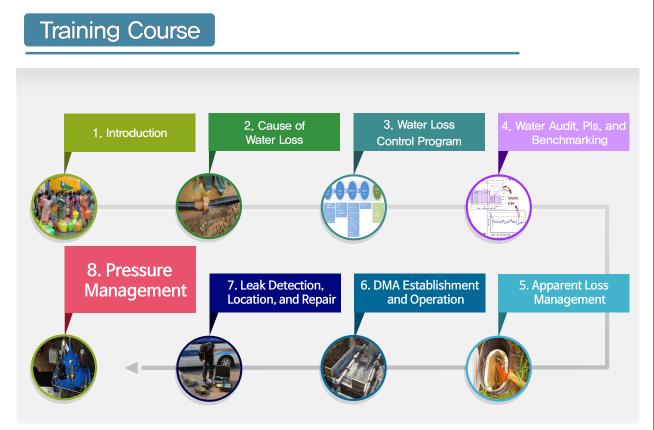
Source : EPA (2010)



Pressure Management

Water Loss Management

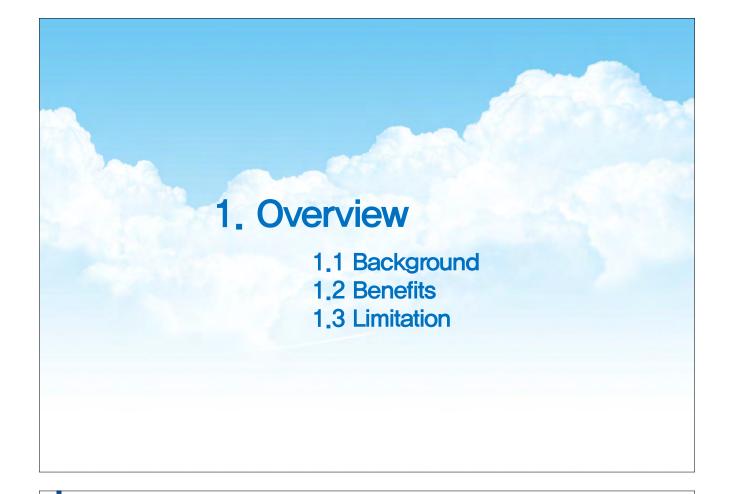




Aims & Objectives
 The course aims to: (1) Provide trainees with detailed procedures for pressure management; (2) Enable trainees to design and implement press management in the water network system
 The objectives are that trainees will understand: (1) The relationship between pressure and leakage; (2) The pros and cons of pressure management; (3) The methods of pressure management

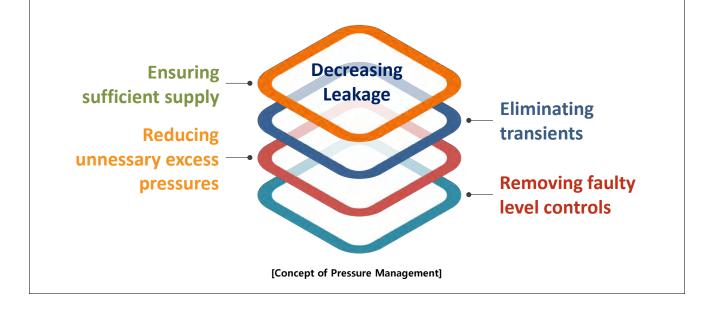
Contents

- 1. Overview
- 2. Pressure Leakage Relationship
- 3. Pressure Management Methods
- 4. Pressure Management Process



1.1 Backgrounds

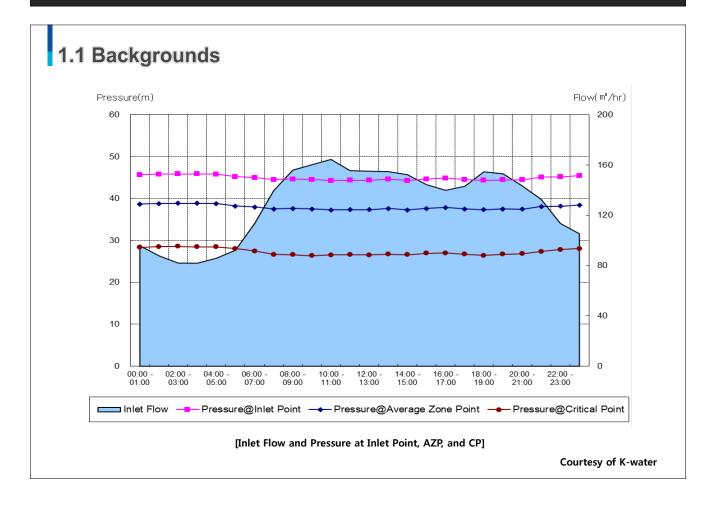
 Definition: Pressure management aims to maintain the optimum levels of service to ensure a sufficient and efficient supply to consumers and, at the same time, reduce unnecessary excess pressures and eliminate transients and faulty level controls, all of which lead to unnecessary leaks in the distribution system



1.1 Backgrounds

- Minimum pressure: The outlet pressure should be sufficient to provide adequate pressure to customers at the most vulnerable prat of the pressure management zone (PMZ)
 - The water utility, water authorities, or local legislation typically define the minimum supply pressure
- Critical point: Notably, the location of the critical point (CP) within a PMZ can be altered depending on variations in consumption behaviors or due to changes in the system structure

 Average zone point (AZP): The AZP is the point in a PMZ that is representative of the average pressure experienced by properties



⁻ This point is referred to as the high ground or it could be at the end of a long, small diameter pipeline

1.1 Background

 Causes of high pressure: In the same PMZ, a high-pressure area occurs due to the following reasons:

Topography and/or distance from supply point

Areas with relatively high pressure will occur due to topography and/or distance from the supply point to ensure sufficient pressure at one critical point

This situation leads to the operation of many parts of a PMZ at pressures significantly higher than required

Different demand patterns

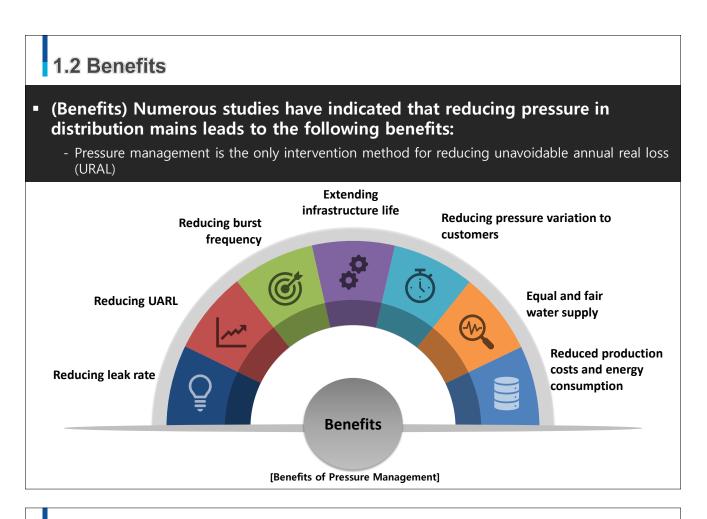
The systems are designed to accommodate pressure and flow requirements during periods with peak demand, which occurs at specific times of the day and during particular months in a year

This tendency results in the operation of a PMZ at pressures significantly higher than required of other periods

[Main Causes of High Pressure]

1.1 Background

- Starting point for managing real loss: Pressure management may be a good starting point for water utilities with high levels of leakage due to the relatively high savings and short payback periods
- Difficulty of implementing in developing countries: Implement pressure management in developing countries is difficult because the network and operation conditions are inadequate, and local water utilities lack a sufficient understanding of pressure management
 - Before implementing effective pressure management, establishing DMA is required
 - Specialized external advice will be required when a water utility engages in pressure management
- Operating with guess: In developing countries, press management is based on guesstimates provided by experienced plumbers
 - No pressure records are collected. Furthermore, no pressure data loggers are available



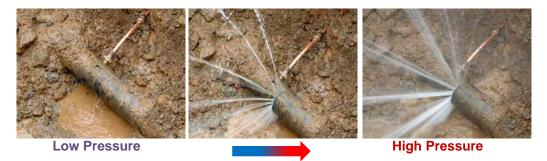
1.3 Limitation

- It cannot be implemented in all sites: Notably, pressure management is not the answer to every case, although it is often one of the most cost-effective measures for leakage reduction
- It only alleviates: Water utilities should note that pressure management only alleviates the impacts but not cure the causes of real loss
- Caution is required: Even if pressure is only reduced to a level that still meets the requirements of domestic and commercial users, many appliances might be affected, such as instantaneous hot water systems, fire sprinkler stems, or home dialysis systems



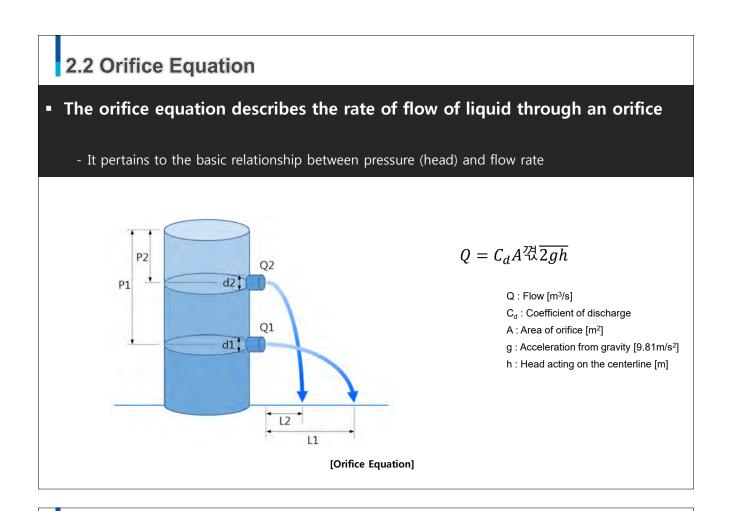
2.1 Pressure versus Leakage

- Relationship: Pressure–leakage relationships are a complex issue. However, a utility can assume a roughly linear relationship between the two
- Multiple relationships: Intensive research and trials have provided firm evidence that multiple relationships occur between leakage and pressure



[Relationship Between Average Zone Pressure and Leakage Index]

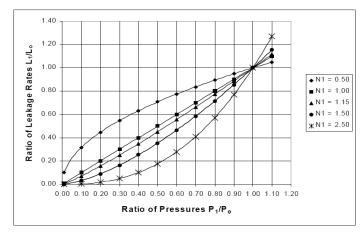
Source: IWA (2014)



2.3 Fixed and Variable Area Discharge (FAVAD)

 FAVAD: Many theories have been postulated to explain pressure-leakage relationships within municipal water supply systems with the FAVAD as the most widely accepted approach

- It postulates that leakage on the network will occur through a mixture of homes, many of which will have paths that do not vary with pressure, whereas other homes will have paths that vary with pressure



$$\frac{L_1}{L_0} = \mathfrak{G} \frac{P_1}{P_0} \mathfrak{G}^{N1}$$

L₀ : Initial leak flow rate at pressure P₀

 L_1 : Leak flow rate at adjusted pressure P_1

- P₀: Initial average zone pressure
- P₁ : Adjusted average zone pressure
- N1 : Leakage exponent

[Interaction Between Pressure and Leakage for Different Values of N1] Source : Lambert et al. (2001) "What Do We Know About Pressure Leakage Relationships in Water Distribution Systems?"

2.3 Fixed and Variable Area Discharge (FAVAD)

Leakage exponent N1: It is used to calculate the relationship between pressure and leakage

- Pressure management in developing countries is more important because they use more plastic pipes, which is more sensitive to pressure



Fixed area discharge

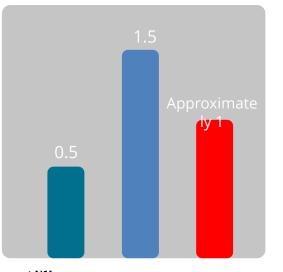
In the case of leaks from steel/iron pipes, the size of the leakage path (i.e., hole) remains constant during the change in pressure

Variable area discharge

In the case of leaks from plastic pipes or cracks in asbestos cement pipes, the surface area of the leakage path changes when pressure changes

Mixed discharge

The split will depend on the proportion of steel/iron pipes to plastic/asbestos pipes. The relationship is complex. However, utility managers should initially assume a linear relationship (i.e., N1 = 1)



[Value of Leakage Exponent N1]

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3.1 Methods

Methods: To manage pressure in PMZ, several methods can be implemented



CREATING A PRESSURE MANAGEMENT ZONE

Creating a pressure management zone is more important than any pressure controller



VARIABLE SPEED PUMP **CONTROL**

In networks with direct pump infeed, variable speed pumps with intelligent control systems can be used to reduce excessive pressures during off-peak periods of the day



PRESSURE-REDUCING VALVE

The modern equivalent of break pressure tank is the pressure-reducing valve (PRV)



REDUCING PUMP HEAD

This method is applicable if the reduced level of pumping can maintain the required minimum pressure



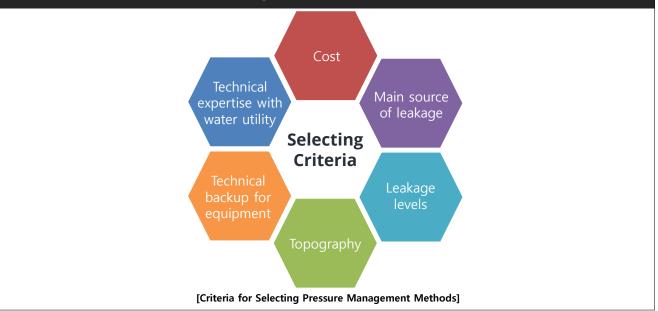
BREAK PRESSURE TANKS

Traditionally, water utilities will construct break pressure tanks and effective small service reservoirs to reduce pressure

[Pressure Management Methods]

3.1 Methods

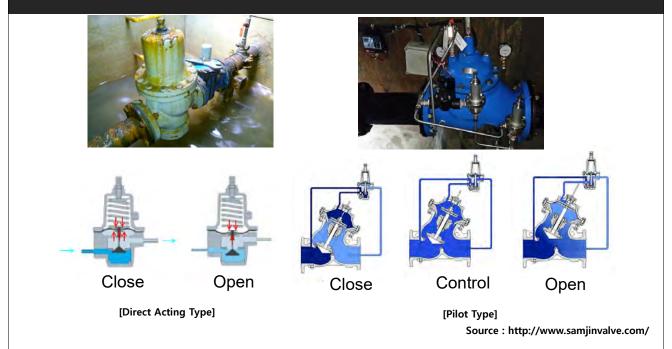
 Selecting methods: Selecting the most appropriate form of pressure control will be based on many factors, such as cost, technical expertise with the water utility, technical backup for the equipment, topography, leakage levels, and the main source of leakage

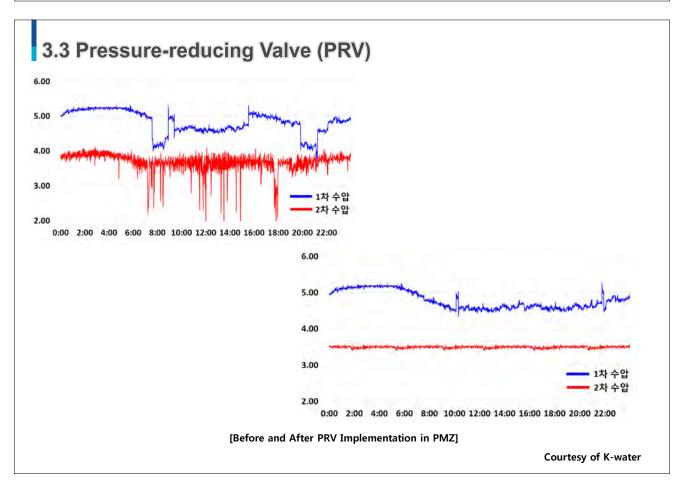


3.2 Establishing the Pressure Management Zone

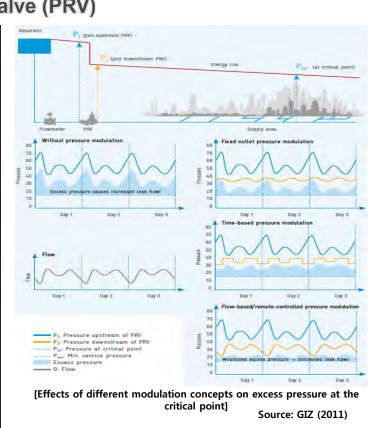
- This is the most difficult task in the pressure management process: Installing and operating controllers are easy parts of this intervention, whereas establishing an appropriate pressure management zone is the most difficult and time-consuming part of the work
- Selecting the PMZ: Selecting districts for pressure reduction should be combined with the audit of existing districts and setting up of new ones
- PMZ and DMA: Typically, DMAs and PMZs are identical
 - However, if the difference of elevation or change in demand patterns in a DMA is large, then the DMA can be divided into several small PMZs

 A variety of PRVs are available, and suppliers should provide training in setting up and maintaining each PRV



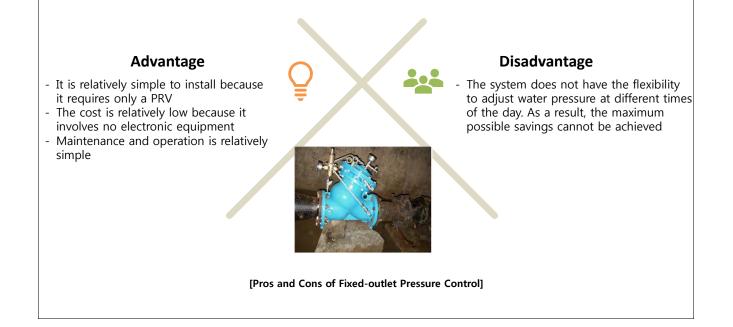


- PRV Control Types: Three basic methods of operating the PRV and modulating system pressure are available:
 - Fixed-outlet pressure control
 - Time-modulated pressure control
 - Flow-modulated pressure control



3.3 Pressure-reducing Valve (PRV)

 Fixed-outlet pressure control: The PRV's outlet pressure is maintained at the selected level at all times. It is the most basic pressure control system



 Time-modulated pressure control: This method is conducted using a PRV similar to that of the fixed-outlet control. However, it varies the pressure setting over a 24-hour period

Advantage

- The controller provides increased flexibility by allowing pressure to be reduced at specific times of the day, which results in greater savings
- The controller is relatively easy to set up and operate
- The installation does not require a flow meter because the controller connects directly to the pilot on the PRV



Disadvantage

- It does not react to the demand for water, which can be a problem if a fire breaks out and requires full pressure for firefighting
- The time-modulated option is more expensive than the fixed-outlet option

[Pros and Cons of Time-modulated Pressure Control]

3.3 Pressure-reducing Valve (PRV)

 Flow-modulated pressure control: Different outlet pressures can be set for different flow rates to maintain the minimum required pressure in the zone during peak flow. The PRV can be opened when the threshold flow is exceeded (e.g., fire flow)

Advantage It provides greater control and flexibility than the time-modulated option It will provide greater savings than the two other options One key advantage is that the flow-modulated option will not hamper the water supply in the case of fire

[Pros and Cons of Flow-modulated Pressure Control]

Control type selection: To select the optimal pressure control method, a comprehensive review of the following factors should be considered before decision making.

Factors	Conditions		
	Fixed outlet	Time-based	Flow-based
① Is there a large change in the flow?	Small	Big	Big
2 Does the change in pressure at the CP occur due to change in flow rate?	No	Yes	Yes
3 Between the pressure and flow at the inlet point, which has more effect on the pressure pattern at the CP ?	Inlet pressure	Flow pattern	Flow pattern
④ Does the flow rate change regularly?	Regular/Irregular	Regular	Irregular
(5) Is the CP located upstream or downstream of the DMA?	Mid or Upstream	Downstream	Downstream
Are there large customers near the CP?	No	Yes	Yes
O Are there many head loss from the inlet point to the CP?	Small	Big	Big

[Factors for Selecting the Pressure-reducing Valve Control Method]

Courtesy of K-water

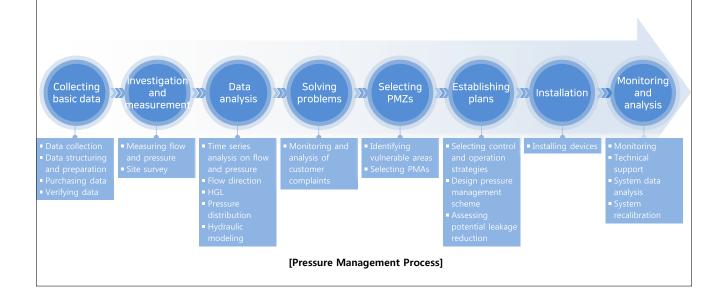
4. Pressure Management Process

- 4.1 Process
- 4.2 Collecting Basic Data
- **4.3 Network Investigation and Measurement**
- **4.4 Flow and Pressure Analyses**
- 4.5 Solving Pressure Problems and Complaint
- **4.6 Selecting Pressure Management Areas**
- **4.7 Establishing Pressure Management Plans**
- **4.8 Installing Pressure Management Systems**
- **4.9 Monitoring and Result Analyses**

4.1 Process

Process: A typical pressure management project is typically composed of the following steps

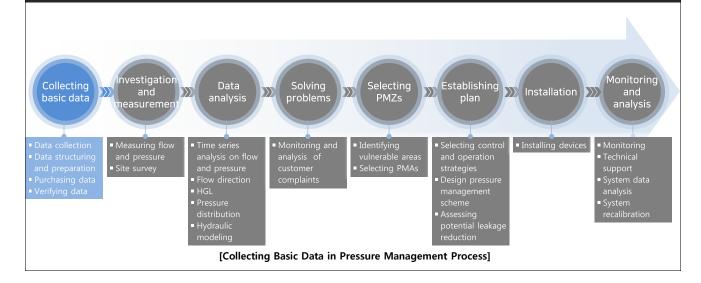
- Although several different types of operating pressure management systems exist, the basic steps and installations are always similar

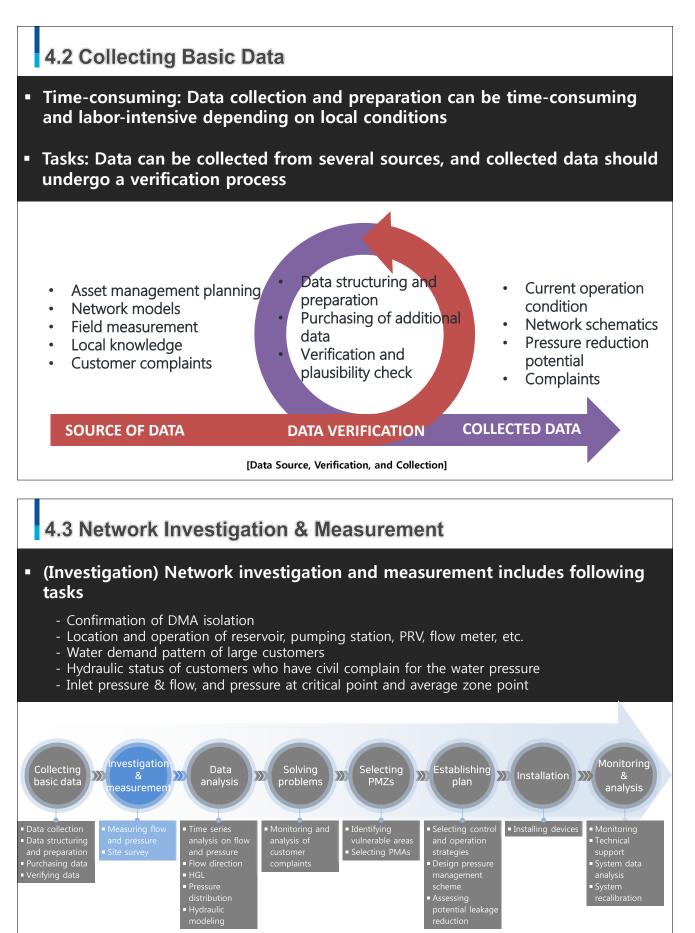


4.2 Collecting Basic Data

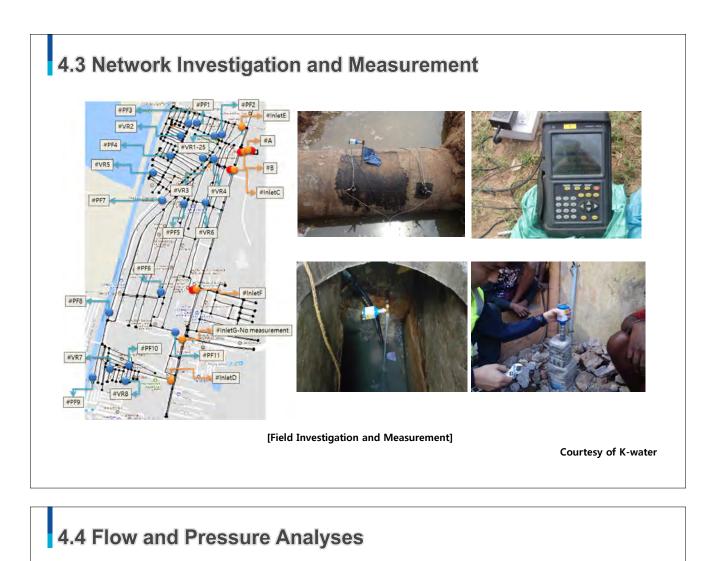
Data collection is necessary to determine the most suitable option and further optimize the system

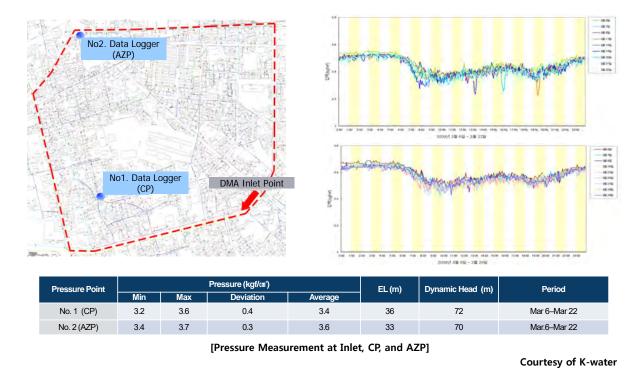
- Data management will be beneficial even if the water utility does not adopt a pressure management program because it provides the water utility with better knowledge and understanding of the network

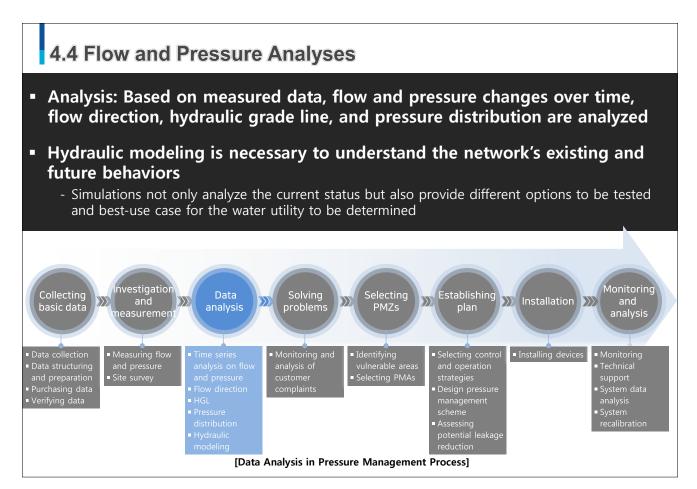


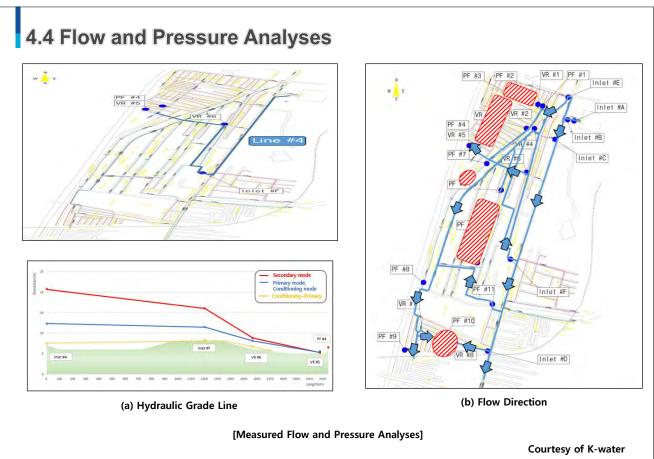


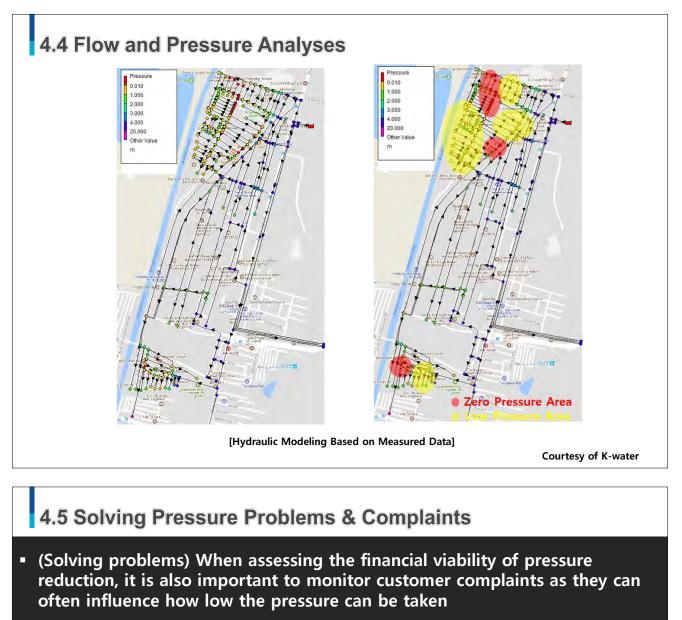
[Investigation and Measurement in Pressure Management Process]



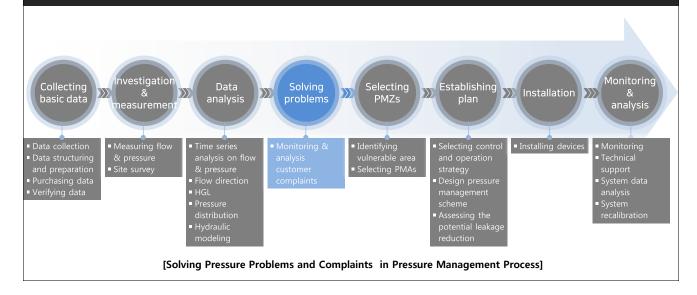






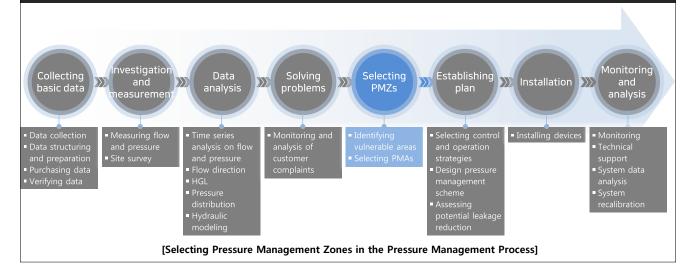


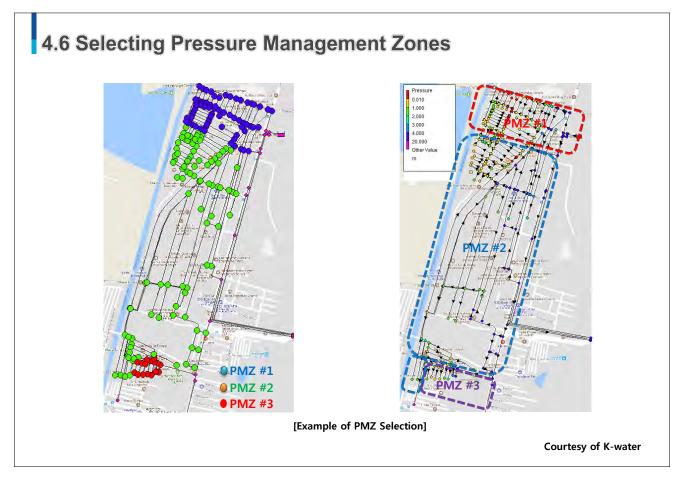
- The issue of pressure management will often come down to some form of subjective decision and trade-off between savings and complaints



4.6 Selecting Pressure Management Zones

- Selection criteria: The potential for the pressure reduction of entire PMAs can be examined by assessing pressure at critical, average zone pressure, and average zone night pressure points in PMZs
- Identification: Before determining the PMZs, areas with excess pressure, wide pressure fluctuation, and low pressure, and newly extended areas should be identified



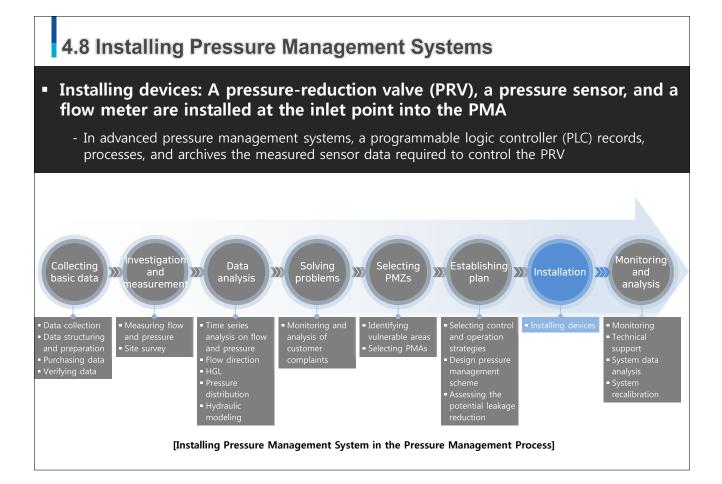


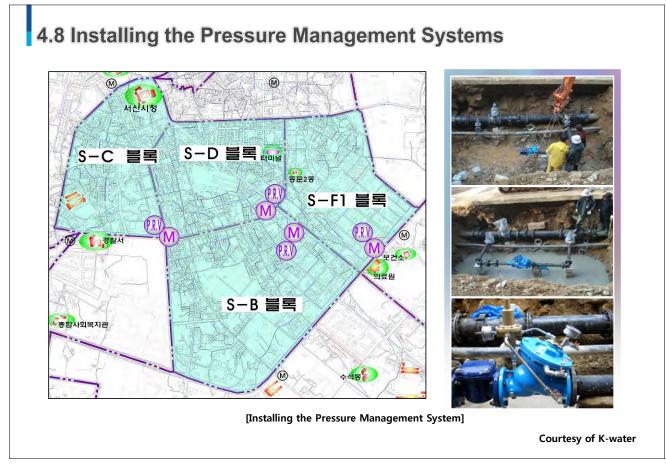
4.7 Establishing Pressure Management Plans

Establishing a plan: A pressure management plan includes a selection of control and operation strategies, design and implementation of the pressure measurement scheme, and assessment of potential leakage reduction Investigation Monitoring Selecting Establishing Installation Collecting Solving problems Data and and basic data analysis analysis neasuremen Identifying vulnerable areas Monitoring and analysis of Data collection
 Data structuring Measuring flow and pressure MonitoringTechnical Time series analysis on flow and pressure Installing devices w direction [Establishing Pressure Management Plan in the Pressure Management Process]

4.7 Establishing Pressure Management Plans

- Selecting control and operational strategy: Selecting schemes for pressure reduction may be conducted by assessing potential schemes throughout a supply zone
 - Selecting the best operation strategy for valves and pumps
 - Simulating a change in the operating mode (from intermittent to continuous water supply)
- Design and implementation of the pressure management scheme: This step involves dimensioning, manufacturing, and installing the pressure management scheme and takes account of technical and engineering issues
 - Dimensioning the system (valves and components)
 - Manufacturing and constructing the pressure management scheme
 - Installing valves and programming control units
 - Setting up the modulation scheme
 - Performing intensive functional tests of the system
 - Putting in place a monitoring system (SCADA technology)
- Assessing potential leakage reduction: Individual schemes can be initially assessed for effectiveness using various methods





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4.9 Monitoring and Result Analyses

 Monitoring and analysis: Online (remote) or on-site system monitoring entails technical support and troubleshooting, system data analysis, and system recalibration in the case of changes in boundary conditions

